TRANE TECHNOLOGIES

AMERICAN STANDARD CONDENSER (GROUND MOUNTED) VALID FOR USE INSIDE AND OUTSIDE THE HVHZ (SEE LIMITATIONS HEREIN)

NON-SITE-SPECIFIC STRUCTURAL PERFORMANCE EVALUATION. A DESIGN PROFESSIONAL SHALL BE RESPONSIBLE FOR CERTIFYING THE APPLICATION OF THIS INFORMATION TO ANY SITE-SPECIFIC LOCATION.

NOTE: THIS EVALUATION CERTIFIES BOTH UNIT INTEGRITY AND ANCHORAGE TO HOST STRUCTURE FOR WIND RESISTANCE (FOR AT-GRADE USE ONLY). SEE LIMITATIONS HEREIN.

BACKUP CLIP CLIPPED TO THE BASE (1) #12-14 SHEET METAL ŚCREW PER CLIP "BACKUP CLIP" ATTACHMENT

NOTE: SEE NEXT PAGE FOR ADDITIONAL PRODUCT INSTALLATION DETAILS AND ANCHOR SCHEDULE TABLE.

KEEP CLEAR BASE TAB BRACKET BACKUP CLIP BASE TAB BRACKET BASE TAB BRACKET KEEP CLEAR (NO BRACKETS) BACKUP CLIP BASE TAB BRACKET

(1) BASE CLIP PER CORNER

LAYOUT A

GENERAL NOTES

- THIS SYSTEM HAS BEEN DESIGNED AND SHALL BE FABRICATED IN ACCORDANCE WITH THE REQUIREMENTS OF THE FLORIDA BUILDING CODE EIGHTH EDITION (2023) & ASCE 7-22. THIS SYSTEM MAY BE USED WITHIN AND OUTSIDE THE HIGH VELOCITY HURRICANE ZONE (HVHZ). THIS DESIGN IS NOT INTENDED TO CERTIFY IMPACT RESISTANCE OF THE MECHANICAL UNIT
- DESIGN & CERTIFICATION OF THE UNIT CABINETRY IS APPROVED THROUGH TEST REPORT#0708.01-15 BY AMERICAN TEST LAB OF SOUTH FLORIDA. DESIGN PRESSURES NOTED HEREIN ARE BASED ON MAXIMUM TESTED PRESSURES DIVIDED BY 1.5 SAFETY FACTOR. PRESSURE VALUES IN THIS ARE (ASD) ALLOWABLE DESIGN PRESSURES UNLESS NOTED
- ALL DIMENSIONS AND THE MINIMUM WEIGHT OF MECHANICAL UNIT SHALL CONFORM TO LIMITATIONS STATED HEREIN. ALL MECHANICAL SPECIFICATIONS (CLEAR SPACE, TONNAGE, ETC.) SHALL BE AS PER MANUFACTURER RECOMMENDATIONS AND ARE THE EXPRESS RESPONSIBILITY OF THE
- FASTENERS SHALL BE CADMIUM-PLATED OR OTHERWISE CORROSION-RESISTANT MATERIAL AND SHALL COMPLY WITH "SPECIFICATIONS FOR ALUMINUM STRUCTURES" SECTION J.3.7.2 BY THE ALUMINUM ASSOCIATION INC., AND ANY APPLICABLE FEDERAL, STATE AND OR LOCAL CODES. REFER TO FASTENER MANUFACTURER'S PUBLISHED DATA SHEETS AND RECOMMENDATIONS FOR FASTENER INSTALLATION INSTRUCTIONS.
- ALL CONCRETE SPECIFIED HEREIN IS NOT PART OF THIS CERTIFICATION. AS A MINIMUM, ALL CONCRETE SHALL BE STRUCTURAL CONCRETE 4" MIN. THICK AND SHALL HAVE MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI, UNLESS
- THE CONTRACTOR IS RESPONSIBLE TO INSULATE ALL MEMBERS FROM DISSIMILAR MATERIALS TO PREVENT ELECTROLYSIS.

TERMINOLOGY:

THE FOLLOWING ABBREVIATIONS MAY APPEAR IN THIS APPROVAL:

"ADDTL." FOR "ADDITIONAL", "AHJ" FOR "AUTHORITY HAVING JURISDICTION" "ALUM" FOR "ALUMINUM, "ASD" FOR "ALLOWABLE STRESS DESIGN", "BO" FOR "BUILD-OUT", "CS" FOR "CARBON STEEL", "EA." FOR "EACH", "E.D."/"EDGE"/"EDGE DIST." FOR "EDGE DISTANCE", "ELEV" FOR "ELEVATION", "EMBED" FOR "EMBEDMENT", "EQ"/"EQUIV." FOR "EQUIVALENT", "EXT" FOR "EXTERIOR", "FBC" FOR "FLORIDA BUILDING CODE", "ft" OR " ' " FOR "FEET", "G" FOR "SPECIFIC GRAVITY", "GA" FOR "GAUGE", "GALV" FOR "GALVANIZED", "GFB" FOR "GROUT-FILLED BLOCK", "GR" FOR "GRADE", "HOLLOW" FOR "HOLLOW BLOCK "HORIZ" FOR "HORIZONTAL", "HVHZ" FOR "HIGH-VELOCITY HURRICANE ZONE", "in OR " " " FOR "INCHES", "INT" FOR "INTERIOR", "KSI" FOR "1,000 lb / in2", "L" FOR "LENGTH", "LB" FOR "POUND", "MAX" FOR "MAXIMUM, "MIN" FOR "MINIMUM", "N.T.S." FOR "NOT TO SCALE", "O.C." FOR "ON-CENTER", "P.E." FOR "PROFESSIONAL ENGINEER", "PERP" FOR "PERPENDICULAR", "PSF" FOR "POUNDS PER SQUARE FOOT (lb/ft2)", "PSI" FOR "POUNDS PER SQUARE INCH (lb/in2)", "QTY" FOR "QUANTITY", "REF." FOR "REFERENCE", "SCHED." FOR "SCHEDULE", "SDS" FOR "SELF-DRILLING SCREWS", "SMS" FOR "SHEET METAL SCREWS", "SPECS" FOR "SPECIFICATIONS", "SS" FOR "STAINLESS STEEL", "SUB" FOR "SUBMITTAL", "TAS" FOR "TESTING APPLICATION STANDARD", "TYP." FOR "TYPICAL", "ULT" FOR "ULTIMATE LOADS" "U.N.O." FOR "UNLESS NOTED OTHERWISE", "UTS" OR "Fu" FOR "ULTIMATE TENSILE STRENGTH/STRESS", "VERT" FOR "VERTICAL", "WLL" FOR "WORKING LOAD LIMIT", "W/" FOR "WITH", "W/O" FOR "WITHOUT", "YS" FOR "YIELD STRENGTH", "#" FOR "NUMBER", "&" FOR "AND", AND "Ø" FOR "DIAMETER".

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NOTE REGARDING USE OF THIS DOCUMENT & USE OUTSIDE FLORIDA:

FRANK BENNARDO, P.E.

PE# 0046549 CA# 9885

NON-SITE-SPECIFIC STRUCTURAL PERFORMANCE EVALUATION. THIS PRODUCT EVALUATION IS VALID FOR USE IN **FLORIDA**ONLY. USE OF THIS EVALUATION REQUIRES A REVIEW & CERTIFICATION BY A LOCAL DESIGN PROFESSIONAL WHO SHALL BE RESPONSIBLE FOR THE PROPER ADAPTATION OF THIS GENERAL PERFORMANCE EVALUATION TO ANY SITE-SPECIFIC PROJECT. CONTACT THIS OFFICE AT ENGINEERINGEXPRESS.COM/QUOTE FOR ASSISTANCE WITH YOUR PROJECT-SPECIFIC NEEDS & FOR ADAPTATION & CERTIFICATION OF THIS DOCUMENT OUTSIDE OF FLORIDA.

ELECTRICAL GROUND, WHEN REQUIRED, TO BE DESIGNED & INSTALLED BY OTHERS.

- THE ADEQUACY OF ANY EXISTING STRUCTURE TO WITHSTAND SUPERIMPOSED LOADS SHALL BE VERIFIED BY THE ONSITE DESIGN PROFESSIONAL AND IS NOT
- BASEPAN MATERIAL CHOPPED FIBER LAMINATE W/ Fy=15 KSI. PLASTIC COMPONENTS USED WITHIN THE HVHZ MUST MEET ALL APPLICABLE FIRE/SMOKE/UV PERFORMANCE REQUIREMENTS AS SET FORTH IN THE ABOVE-NOTED BUILDING CODE.
- THE SYSTEM DETAILED HEREIN IS GENERIC AND DOES NOT PROVIDE INFORMATION FOR A SPECIFIC SITE. FOR SITE CONDITIONS DIFFERENT FROM THE CONDITIONS DETAILED HEREIN, A LICENSED ENGINEER OR REGISTERED ARCHITECT SHALL PREPARE SITE-SPECIFIC DOCUMENTS FOR USE IN CONJUNCTION WITH THIS DOCUMENT.
- WATER-TIGHTNESS OF EXISTING HOST SUBSTRATE SHALL BE THE FULL RESPONSIBILITY OF THE INSTALLING CONTRACTOR. CONTRACTOR SHALL ENSURE THAT ANY REMOVED OR ALTERED WATERPROOFING MEMBRANE IS RESTORED AFTER FABRICATION AND INSTALLATION OF STRUCTURE PROPOSED HEREIN. THIS ENGINEER SHALL NOT BE RESPONSIBLE FOR ANY WATERPROOFING OR LEAKAGE ISSUES WHICH MAY OCCUR AS WATER-TIGHTNESS SHALL BE THE FULL RESPONSIBILITY OF THE INSTALLING CONTRACTOR.
- ONLY. USE OF THIS SPECIFICATION BY CONTRACTOR, et. al. INDEMNIFIES & SAVES HARMLESS THIS ENGINEER FOR ALL COST & DAMAGES INCLUDING LEGAL FEES & APPELLATE FEES RESULTING FROM MATERIAL FABRICATION SYSTEM ERECTION, & CONSTRUCTION PRACTICES BEYOND THAT WHICH I CALLED FOR BY LOCAL, STATE & FEDERAL CODES & FROM DEVIATIONS OF THIS
- EXCEPT AS EXPRESSLY PROVIDED HEREIN, NO ADDITIONAL CERTIFICATIONS OR AFFIRMATIONS ARE INTENDED.
- ALTERATIONS, ADDITIONS, AND OTHER MARKINGS TO THIS DOCUMENT ARE NOT PERMITTÉD AND INVALIDATE THIS CERTIFICATION

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FOR SITE-SPECIFIC DEVIATIONS & MORE INFORMATION ABOUT THIS DOCUMENT OR SCAN THIS QR CODE

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AMERICAN STANDARD CONDENSER (GROUND MOUNTED)
FLORIDA BUILDING CODE EIGHTH EDITION (2023)
FLORIDA STATEWIDE APPROVAL (FSA)

TECHNOLOGIE

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TROUP

23-68282 SCALE: NTS UNLESS NOTED

0.800"

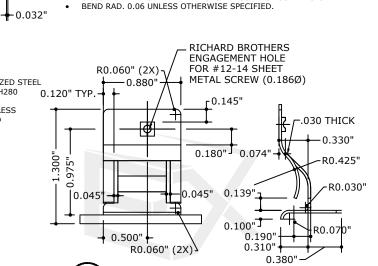
+0.400" Ø0.250" 00'0 0.00 Ø0.250" Ø0.250" R0.250' -00.2500.187" R0.250 BACKUP CLIP MATERIAL: R0.060" (2X) 1050 STEEL 0.030 THK 1.50 0.0002 THK. ZINC PLATE WITH CRONAK SURFACE CONVERSION. r0.200"

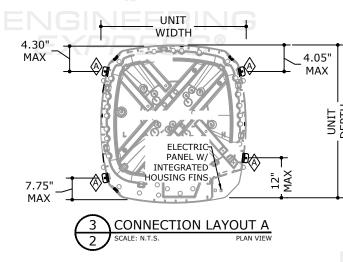
0 BASE TAB BRACKET MATERIAL: PRE-PAINTED GALVANIZED STEEL 0.06 ES3101203, PER B50MH280 .060 THK. .060 INSIDE RADII UNLESS OTHERWISE SPECIFIED

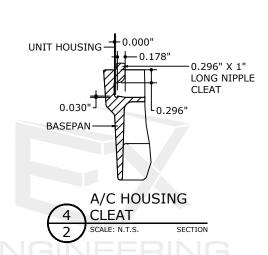
D С Ε LAYDUT D153086P01 2.50 2.32 | 2.43 | 3.33 | 3.78 | 1.48 D153086P02 2.10 1.91 2.03 2.93 3.38 1.08

BASE TAB BRACKETS SCALE: N.T.S.

^LR0.120" TYP.







FRANK BENNARDO, P.E.

PE# 0046549 CA# 9885

BENNA

UNIT MODEL



<u>APPLICABLE MODELS</u>: 2/4A6B, 4A6C, 4A6H, 4A6Z, 4A6V, 2/4A7B, 2/4A7C, 4A7A, 4A7Z, 4A7V, 2TTB3, 2TWB3, 4TTB3, 4TTB6, 4TWB3, 4TWB4, 5A6C, 5A6H, 5A6V, 5A7A, 5A7C, 5A7V

BACKUP CLIP

ALLOWABLE WIND PRESSURES FOR MULTIPLE BASE BRACKET AND UNIT HOUSING CONFIGURATIONS				ALLOWABLE (ASD) WIND PRESSURE	APPLICABLE ANCHOR TYPES: SEE ANCHOR NOTES FOR A DESCRIPTION OF ANCHOR	LAYOUT OF BASE TAB BRACKETS:
DESIGNATION	UNIT WIDTH	UNIT DEPTH	UNIT HEIGHT	<u>RATING</u>	TYPE #1 OR #2	
TYPICAL UNIT	25.69 IN	28.42 IN	32.77 IN	60 PSF	TYPE 1 & TYPE 2	LAYOUT A
TYPICAL UNIT	25.69 IN	28.42 IN	25.60 IN	60 PSF	TYPE 1 & TYPE 2	LAYOUT A
TYPICAL UNIT	29.69 IN	32.65 IN	40.70 IN	60 PSF	TYPE 1 & TYPE 2	LAYOUT A
TYPICAL UNIT	29.69 IN	32.65 IN	28.77 IN	60 PSF	TYPE 1 & TYPE 2	LAYOUT A
TYPICAL UNIT*	34.29 IN	37.29 IN	45.17 IN	60 PSF	TYPE 1 & TYPE 2	LAYOUT A
TYPICAL UNIT	34.29 IN	37.29 IN	29.18 IN	60 PSF	TYPE 1 & TYPE 2	LAYOUT A

*UNIT WEIGHT = 245 LB (DEFAULT UNIT WEIGHT = 120 LB)

ANCHOR NOTES:

- 1. SEE ISOMETRIC BASE LAYOUT (PREVIOUS PAGE) FOR ANCHOR LOCATIONS AND/OR SPACING.
- 2. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS' RECOMMENDATIONS. UTILIZE 0.625" O.D. x 0.280" I.D. x 0.059" THICK WASHER @ BASE CLIP.
- 3. ANCHOR TYPE #1: CONSIDERS HILTI KWIK-CON II+CARBON STEEL TAPCONS OR EQUIVALENT W/ 1-3/4" MIN EMBED, 2-1/2" MIN EDGE DISTANCE AND 3" MIN SPACING (UNLESS NOTED OTHERWISE), FASTENED TO MINIMUM 3,000 PSI EXISTING CONCRETE AS VERIFIED
- 4. ANCHOR TYPE #2: CONSIDERS SHEET METAL SCREWS (SMS) AS SPECIFIED HEREIN SHALL BE MINIMUM - SAE GRADE 5 ASTM A449 - SPACED THREAD W/ MIN (5) PITCHES PAST THREAD PLANE, INTO MINIMUM 1/8" THICK A36 STEEL. USE #14-14 SMS SCREWS WITH 5/8" EDGE DISTANCE FOR STEEL HOST STRUCTURE.
- 5. MINIMUM EMBEDMENT SHALL BE AS NOTED. MINIMUM EMBEDMENT AND EDGE DISTANCE EXCLUDES STUCCO, FOAM, INSULATION, AND OTHER FINISHES.

TABLE DIRECTIONS:

- 1. SELECT DESIRED UNIT SIZE.
- 2. SELECT APPLICABLE ANCHOR TYPE UNDER CONSIDERATION AS VERIFIED BY OTHERS.
- MATCH UNIT SIZE WITH THE INTENDED HOST STRUCTURE AND OBSERVE MAXIMUM ALLOWABLE WIND PRESSURE FOR THE SYSTEM. SITE-SPECIFIC REQUIRED WIND PRESSURES PER SEPARATE CERTIFICATION OR BY OTHERS.
- UTILIZE LAYOUT AS LISTED IN TABLE.

E-DOWN CONFIGURATIONS REQUIRE BRACKET PER CORNER. SEE DETAIL 3/2 FOR ILLUSTRATION.

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APPENDIX A: DESIGN WIND PRESSURE GUIDE

Max. Ult. Wind	Max. MRH (Roof	Exposure	Required Design Wind Pressures (ASD)	
Speed (Vult)	Height)	Category	Lateral Pressure	Uplift Pressure
140	At-Grade	С	± 26 psf	O* psf
140 mph	(0 ft)	D	± 31 psf	0* psf
175	At-Grade	С	± 40 psf	0* psf
175 mph	(0 ft)	D	± 49 psf	0* psf
100	At-Grade	С	± 46 psf	0* psf
186 mph	(0 ft)	D	± 54 psf	0* psf

Note: Any table values with the format shown left, if present, indicate design wind pressures and site conditions that are not approved for use by this evaluation. Seek additional engineering or contact this firm for design solutions.

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FOR DESIGN AID CALCULATORS AND RESOURCES RELATED TO THIS TER & GUIDES HEREIN, OR SCAN THE QR CODE RIGHT >



DIRECTIVE: This design pressure guide is for reference only and shall be approved for use by the Authority Having Jurisdiction (AHJ). If the design pressures listed in this guide are not used, required design pressures shall be calculated separately. For site-specific scenarios classified as Exposure Category B, the required design pressures stated for Exposure Category C in the above guide shall be used or design pressures shall be calculated separately. For heights and parameters beyond the parameters listed in this guide, visit our Online Calculator via the website link (https://ecalc.io/forces) or QR Code below, or obtain calculations separately by others.

The required ASD design pressures listed in this guide were calculated per the table's listed corresponding site conditions. The project design professional or permitting contractor shall verify that the site-specific conditions are equal to or less than the approved design parameters listed in the guide. Per the note below table: any values shown as "XX psf", indicate wind pressures and corresponding site conditions that are not valid for use with this evaluation (exceeds the max. rated pressures).

*Note: Per the codes and standards referenced herein, uplift is not required for mechanical equipment at-grade. If uplift at-grade is required by the AHJ, contact this firm for a site-specific evaluation.

At-Grade (0 ft MRH) Required Design Pressures:

- ASCE 7 "Design Wind Loads: Other Structures"
- Structure Shape = Square, flat terrain
- Height of structure (unit + stand or curb, if used) = 6 ft max.
- Width of unit = 1 ft min., Depth of unit = 11 in min.

UNIT REACTIONS FROM WIND GUIDE

DIRECTIVE: This guide is intended for use by a design professional. Design parameters shall abide all specifications and limitations stated in this report. Design professional shall consider all forces, including seismic and snow loads, per the governing building code. Unit reactions obtained from this guide shall be verified by a registered Professional Engineer. Reactions are applicable for unit-to-host connections only. Sample calculations are provided below.

D/2 ---

Design Parameters:

- Lateral Wind Pressure, P lat
- Unit Height, H
- Unit Width, W
- Support Spacing across Depth, sd
- Uplift Wind Pressure, P up
- Unit Depth, D
- Unit Weight, Wt
- Support Spacing across Width, sw

Unit Reaction Equations:

Long Side (Width x Height):

- Sliding Force, L = P lat x W x H
- Uplift Force, U = P up x W x DTotal Tension per Long Side =
- (LxH/2 + Uxsd/2 Wtx0.6xsd/2)/sd

Short Side (Depth x Height):

- Sliding Force, L = P lat x D x H
- Uplift Force, U = P up x W x D
- Total Tension per Short Side =
- $(L \times H/2 + U \times sw/2 Wt \times 0.6 \times sw/2)/sw$

Example: A (48" W x 36" D x 42" H), 250 lb net weight unit at wind pressures of 120 psf lateral and 95 psf uplift, on a 24" wide roof stand, shall have the following unit reactions:

Long Side (Width x Height):

- Sliding Force, L = P lat x W x H = $(120 \text{ psf}) \times (48 \text{ in}) \times (42 \text{ in}) \times (1 \text{ in}^2 / 144 \text{ ft}^2) = 1680 \text{ lb}$
- Uplift Force, U = P up x W x D = $(95 \text{ psf}) \times (48 \text{ in}) \times (36 \text{ in}) \times (1 \text{ in}^2 / 144 \text{ ft}^2) = 1140 \text{ lb}$
- 3. Total Tension per Long Side =
 - $= (L \times H/2 + U \times sd/2 Wt \times 0.6 \times sd/2)/sd$

$= ((1680 \text{ lb} \times 42/2 \text{ in}) + (1140 \text{ lb} \times 24/2 \text{ in}) (250 \text{ lb} \times 0.6 \times 24/2 \text{ in})) / 24 \text{ in} = 1965 \text{ lb}$

Short Side (Depth x Height):

- 1. Sliding Force, L = P lat x D x H
- = (120 psf) x (36 in) x (42 in) x (1 in2/ 144 ft2) = 1260 lb
- 2. Uplift Force, U = P_up x W x D
- = $(95 \text{ psf}) \times (48 \text{ in}) \times (36 \text{ in}) \times (1 \text{ in}^2/144 \text{ ft}^2) = 1140 \text{ lb}$
- 3. Total Tension per Short Side =
- = (LxH/2 + Uxsw/2 Wtx0.6xsw/2)/sw
- $= ((1260 \text{ lb} \times 42/2 \text{ in}) + (1140 \text{ lb} \times 48/2 \text{ in}) -$

 $(250 \text{ lb} \times 0.6 \times 48/2 \text{ in}))/48 \text{ in} = 1046 \text{ lb}$

IN ALL CONDITIONS IT IS THE RESPONSIBILITY OF THE PERMIT HOLDER TO ENSURE THE HOST STRUCTURE IS CAPABLE OF WITHSTANDING THE RATED GRAVITY, LATERAL, AND UPLIFT FORCES BY SITE-SPECIFIC DESIGN. NO WARRANTY OF ANY KIND, EXPRESSED OR IMPLIED, IS OFFERED BY ENGINEERING EXPRESS AS TO THE INTEGRITY OF THE HOST STRUCTURE TO CARRY DESIGN FORCE LOADS INCURRED BY THIS UNIT.





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