

Project No.:	28227.05-107-16
Project Name:	Energy Saver Steel Wood Edge
	Glazed X
Date:	3/21/2024
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# **PRODUCT APPROVAL SUPPORTING CALCULATIONS**

Series/Model: Energy Saver Steel Wood Edge Glazed X

REPORT NO.: 28227.05-107-16

- RENDERED TO: Jeld-Wen Windows & Doors 3737 Lakeport Blvd Klamath Falls, Oregon
- PREPARED BY: Michael D. Stremmel, P.E. Molimo, LLC 1410 Eden Road York, Pennsylvania 17402

DATE: 3/21/2024

This item has been digitally signed and sealed by Michael D. Stremmel, PE on the date adjacent to the seal.

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Michael D. Stremmel, P.E. Senior Project Engineer FL PE 65868 FL REG 37122

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#### SCOPE:

Molimo, LLC was contracted by Jeld-Wen Windows & Doors to evaluate alternate installation methods for their Energy Saver Steel Wood Edge Glazed X. The evaluation is based on physical testing and product certifications.

Reference standards utilized in this project include:

Florida Building Code. International Code Council.

ANSI/AWC National Design Specification (NDS) for Wood Construction. American Wood Council.

AISI S100 North American Specification for the Design of Cold-Formed Steel Structural Members. American Iron and Steel Institute.

ICC-ES Report ESR-1976 ITW Buildex TEKS Self-Drilling Fasteners. ICC Evaluation Service.

NOA 21-0201.06 *Tapcon Concrete and Masonry Anchors with Advanced Threadform Technology*. Miami-Dade County Product Control Section.

The anchorage analysis presented herein does not address the water resistance, water penetration, or air infiltration performance of the installation method or the installed product. In addition, the analyses rely on the assumption that the building substrate is capable of withstanding the incurred loads.



## Certification of Independence

In accordance with Rule 61G20-3 Florida Administrative Code, Molimo, LLC hereby certifies the following:

- Molimo, LLC does not have, nor does it intend to acquire or will it acquire, a financial interest in any company manufacturing or distributing products tested or labeled by the agency.
- Molimo LLC s is not owned, operated or controlled by any company manufacturing or distributing products it tests or labels.
- Michael D. Stremmel, P.E. does not have nor will acquire, a financial interest in any company manufacturing or distributing products for which the reports are being issued.
- Michael D. Stremmel, P.E does not have, nor will acquire, a financial interest in any other entity involved in the approval process of the product.



#### ANALYSES:

## Summary of Test Results

Table 1 summarizes the various Energy Saver Steel Wood Edge Glazed X products and their corresponding performance levels which have been established by testing or product certification.

Series/Model	Test Report Number	Size (W x H)	Performance
Jeld-Wen Energy Saver Steel Wood Edge Inswing/Outswing Opaque O/X	National Certified Test Lab Report No. SJW2010-001 (2/5/2010)	38" x 82"	+50 / -50 psf

Testing documented in Table 1 was conducted by National Certified Testing Laboratories of Everett, Washington (Florida Department of Business & Professional Regulation Test Lab No. TST9341 – laboratory was approved at the time of testing). The testing documented above is certified by NAMI under certification number NI010126.01-R10 (Expires 1/31/2026).

## **As-Tested Installation Analysis**

For air/water/structural testing, the test specimen was secured to a Douglas-Fir wood test buck with #10 wood screws (1-1/2" min. embedment) at the head, sill, and jambs. The as tested installation method is evaluated on Pages 5 and 6. These capacities will be used to prove acceptable anchors and substrates for the product.

## **Alternate Anchorages**

Calculations on Pages 7 through 13 determine the design capacity of alternate installation anchorages for the product.

## Anchorages Requirements

As-tested spacing must be maintained. It must be determined that the anchorages are not overloaded for the approved product size and design pressures. Calculations presented on Page 14 show the alternate anchorages are acceptable for the established product performance.

Anchorage requirements established by this report are accurately presented in Drawing D1000382.



## <u>As-Tested Installation – Through Frame to Wood</u>

- Anchor: #10 x 2-1/2" Wood Screw (1-1/2" min embedment})
- Details: 0.719" thick wood frame (G = 0.42) No shim space was utilized
- Substrate: Spruce-Pin-Fir wood test buck (G = 0.42)

Wood Screw Capacity (Shear)

Z' = <u>131 lb</u>

(See Following Page)

Design Capacity of the Connection = 131 lb



## Lateral Design Strength of Wood Connections

#### <u>Data</u>

<u>Fastener</u>			
Fastener	=	#10 Wo	ood Screw
Shank Dia	=	0.190	in.
Root Dia.	=	0.152	in.
F <sub>yb</sub>	=	80,000	psi
Fastener length	=	2.500	in.
<u>Main Member</u>			

Main	Men	<u>nber</u>	
N	later	ial	

Material	=	S	PF
G	=	0.42	
θ	=	90	
F <sub>e</sub>	=	3,350	psi
Thickness	=	1.500	in.

## **Calculations**

#### Lateral Bearing Factors

D	=	0.152	in.
$\ell_{\rm m}$	=	1.451	in.
$K_{\theta}$	=	1.25	
K <sub>D</sub>	=	2.20	
$R_{e}$	=	1.000	
R <sub>t</sub>	=	2.02	

#### Project: Energy Saver Steel Wood Edge Glazed X **Comments:** 1-1/2" min embedment

Side Membe	<u>er</u>		
Material	=	S	PF
G	=	0.42	
θ	=	90	
$F_{es}$	=	3,350	psi
Thickness	=	0.719	in.

$\mathbf{k}_1$	=	0.6847	
$k_2$	=	1.1271	
$k_3$	=	1.48	
R <sub>d</sub>	=	2.20	(Mode I <sub>m</sub> , I <sub>s</sub> )
R <sub>d</sub>	=	2.20	(Mode II)
R <sub>d</sub>	=	2.20	(Mode III <sub>m</sub> , III <sub>s</sub> , IV)
r d	-	2.20	$(10000 \text{ m}_{\text{m}}, 11_{\text{s}}, 17)$

#### <u>Lateral Design Values, Z</u>

=	336	lbf
=	166	lbf
=	114	lbf
=	126	lbf
=	82	lbf
=	99	lbf
	= = =	= 166 = 114 = 126 = 82

<== Minimum Value

#### **Adjustment Factors**

C <sub>D</sub> =		1.6	
Wet S	ervic	e Factor	
Fabrication/In-Ser	vice	Dry/Dry	
C <sub>M</sub> =		1.0	
In service tempera	iture	T≤1	LOO°F
C <sub>t</sub> =		1.0	
C <sub>g</sub> =		1.0	
Adjusted Design V	/alue	e, <u>Z</u>	

Z' 131 lbf =

#### $\mathbf{C}_{\Delta}$ 1.0 = Is fastener installed in end grain? No C<sub>eg</sub> 1.00 = Is fastener part of a diaphragm? No $C_{di}$ 1.0 = Is fastener toe-nailed? No $C_{tn}$ 1.00 =



## Alternate Installation – Strap Anchor to Wood

Anchor:	<ul><li>(2) #10 x 1-1/2" Flat head screws securing the 1/4" max shim space</li></ul>	ne strap to the substrate		
Details:	ils: 20 gauge (0.033" thick) 33 KSI steel strap anchor w/ two #10 screws securing th strap to the frame 1-1/2" thick wood frame			
Substrate:	Spruce-Pine-Fir 2x Wood Substrate (G = 0.4)	2 min.)		
<u>Wood Scre</u>	<u>w Capacity</u> (Shear)			
Z' = <u>155</u>	<u>lb</u>	(See Following Page)		
Bending of #10 x 1-1/2" flat head screw				
L = 1/4"	L = 1/4" (maximum shim space)			
S = $\pi$ d <sup>3</sup> / 32 = $\pi$ (0.190") <sup>3</sup> / 32 = 0.000673 in <sup>3</sup>				
$F_b = (1.3)(0.6 F_y) = (1.3)(0.6)(90,000 \text{ psi}) = 70,200 \text{ psi}$ (1.3 for weak axis bendin				
$F_b = M / S = (V) (L/2) / S$ (L/2 for guided bending)				

- V = 2 S F<sub>b</sub> / L = (2)(0.000221 in3)(70,200 psi) / 1/4"
- V = <u>378 lb</u>

Bearing Capacity (of #10 screw on frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.190'')(0.719'')/(10(0.190) + 0.5)$ 

= <u>191 lb</u>per screw

Bearing Capacity (of strap anchor)

 $P_b = 2.7 \text{ D t } F_{tu} = 2.7(0.190")(0.033")(45,000 \text{ psi}) = 761 \text{ lb}$  $P_{allow} = 761 \text{ lb } / 3.0 = 254 \text{ lb per screw}$ 

Design Capacity of the Connection = 155 lb



Energy Saver Steel Wood

1-1/2" min embedment

ASTM A 653, Grade 33 Steel

psi

in.

(Mode  $I_m$ ,  $I_s$ )

Edge Glazed X

N/A

90

61,850

0.033

**Comments:** Steel Strap Installation

## Lateral Design Strength of Wood Connections

SPF

#### <u>Data</u>

<u>Fastener</u>			
Fastener	=	#10 Wo	od Screw
Shank Dia	=	0.190	in.
Root Dia.	=	0.152	in.
$F_{yb}$	=	80,000	psi
Fastener length	=	1.500	in.
<u>Main Member</u>			

n Member		
Material	=	
G	=	0.42
Δ	_	00

θ	=	90	
F <sub>e</sub>	=	3,350	psi
Thickness	=	1.500	in.

## **Calculations**

#### Lateral Bearing Factors

D	=	0.152	in.
$\ell_{\rm m}$	=	1.277	in.
$K_{\theta}$	=	1.25	
K <sub>D</sub>	=	2.20	
R <sub>e</sub>	=	0.054	
R <sub>t</sub>	=	38.70	

#### 

Project:

=

=

=

=

=

Side Member

G

θ

 $F_{es} \\$ 

Material

Thickness

R <sub>d</sub>	=	2.20	(Mode II)
R <sub>d</sub>	=	2.20	(Mode III <sub>m</sub> , III <sub>s</sub> , IV)

1.0

No

#### Lateral Design Values, Z

Mode $I_m$	=	296	lbf
Mode I <sub>s</sub>	=	141	lbf
Mode II	=	121	lbf
$Mode III_m$	=	143	lbf
Mode III <sub>s</sub>	=	97	lbf
Mode IV	=	137	lbf

<== Minimum Value

#### Adjustment Factors

Z'

C <sub>D</sub> =	:	1.6	
Wet S	Servic	e Factor	
Fabrication/In-Se	rvice	Dry/Dry	
C <sub>M</sub> =	:	1.0	
In service tempera	ature	T≤1	LOO°F
C <sub>t</sub> =	:	1.0	
C <sub>g</sub> =	:	1.0	
Adjusted Design Value, Z			

=

155 lbf

	C <sub>eg</sub>	=	1.00
]	Is fastener part of a di	aphragm?	No
	C <sub>di</sub>	=	1.0
	Is fastener to	pe-nailed?	No
	C <sub>tn</sub>	=	1.00

 $\mathbf{C}_{\Delta}$ 

Is fastener installed in end grain?

=



## Alternate Installation – Through-Frame to Concrete

Anchor: 3/16" Tapcon Anchor - 1-1/4" min embedment - 2-1/2" min edge distance - 4" min anchor spacing - 1/4" max shim space Details: Through the Wood Frame - 1" thick Substrate: 3,000 psi Concrete Anchor Capacity (Shear of 3/16" Tapcon)  $P_{ss} / \Omega = 181 lb$ (NOA-No. 16-1222.06) **Bearing Capacity** (of Wood frame)  $P_b = F_e D t / K_D = (3,350 \text{ psi})(0.170'')(1.00'')/(10(0.170) + 0.5) = 259 \text{ lb}$ Bending Capacity (of 3/16" Tapcon) L = 1/4" (maximum shim space) S =  $\pi$  d<sup>3</sup> / 32 =  $\pi$  (0.170")<sup>3</sup> / 32 = 0.000482 in<sup>3</sup>  $F_b = (1.3)(0.6 F_v) = (1.3)(0.6)(137,000 \text{ psi}) = 106,860 \text{ psi}$ (1.3 for weak axis bending)  $F_{b} = M / S = (V) (L/2) / S$ (L/2 for guided bending)  $V = 2 S F_b / L = (2)(0.000482 in3)(106,860 psi) / 1/4"$ V = 412 lb

#### Design Capacity of the Connection = 181 lb



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## Alternate Installation – Through-Frame to CMU Block

Anchor: 3/16" Tapcon Anchor - 1-1/4" min embedment - 2-1/2" min edge distance - 4" min anchor spacing - 1/4" max shim space Details: Through the Wood Frame - 1" thick Substrate: **CMU Block** Anchor Capacity (Shear of 3/16" Tapcon)  $P_{ss} / \Omega = 135 \text{ lb}$ (NOA-No. 16-1222.06) **Bearing Capacity** (of Wood frame)  $P_b = F_e D t / K_D = (3,350 \text{ psi})(0.170'')(1.00'')/(10(0.170) + 0.5) = 259 \text{ lb}$ Bending Capacity (of 3/16" Tapcon) L = 1/4" (maximum shim space) S =  $\pi$  d<sup>3</sup> / 32 =  $\pi$  (0.170")<sup>3</sup> / 32 = 0.000482 in<sup>3</sup>  $F_b = (1.3)(0.6 F_v) = (1.3)(0.6)(137,000 \text{ psi}) = 106,860 \text{ psi}$ (1.3 for weak axis bending)  $F_{b} = M / S = (V) (L/2) / S$ (L/2 for guided bending)  $V = 2 S F_b / L = (2)(0.000482 in3)(106,860 psi) / 1/4"$ V = 412 lb

## Design Capacity of the Connection = 135 lb



## Alternate Installation – Strap Anchor to Concrete

- Anchor: 3/16" TapCon Anchor
  - 1-1/4" min embedment
  - 2-1/2" min edge distance
  - 4" min anchor spacing
  - 1/4" max shim space
- Details: 20 gauge (0.033" thick) 33 KSI steel strap anchor w/ two #8 screws securing the strap to the frame
  - 1.00" thick wood frame
- Substrate: 3,000 psi Concrete

Anchor Capacity (Shear of 3/16" Tapcon)

$$P_{ss} / \Omega = 181 \text{ lb}$$

(NOA-No. 16-1222.06)

Bearing Capacity (of 3/16" Tapcon on strap anchor)

P<sub>b</sub> = 2.7 D t F<sub>tu</sub> = 2.7(0.170")(0.033")(45,000 psi) = 681 lb

P<sub>allow</sub> = 681 lb / 3.0 = <u>227 lb</u>

Bearing Capacity (of #8 screws on frame)

 $P_b = F_e D t / K_D = (3,350 \text{ psi})(0.164'')(1.00'')/(10(0.164) + 0.5) = 257 \text{ lb}$ 

Bearing Capacity (of #8 screws on strap anchor)

P<sub>b</sub> = 2.7 D t F<sub>tu</sub> = 2.7(0.164")(0.033")(45,000 psi) = 657 lb

 $P_{allow} = 657 \text{ lb} / 3.0 = 219 \text{ lb}$ 

Bending Capacity (of 3/16" Tapcon)

L = 1/4" (maximum shim space)  $S = \pi d^3 / 32 = \pi (0.170")^3 / 32 = 0.000482 \text{ in}^3$   $F_b = (1.3)(0.6 F_y) = (1.3)(0.6)(137,000 \text{ psi}) = 106,860 \text{ psi} \qquad (1.3 \text{ for weak axis bending})$   $F_b = M / S = (V) (L/2) / S \qquad (L/2 \text{ for guided bending})$   $V = 2 \text{ S } F_b / \text{ L} = (2)(0.000482 \text{ in}3)(106,860 \text{ psi}) / 1/4"$   $V = \underline{412 \text{ lb}}$ 

Design Capacity of the Connection = 181 lb (one concrete anchor per strap)



## Alternate Installation – Strap Anchor to CMU Block

Anchor: 3/16" Tapcon Anchor - 1-1/4" min embedment - 2-1/2" min edge distance - 4" min anchor spacing - 1/4" max shim space Details: 20 gauge (0.033" thick) 33 KSI steel strap anchor w/ two #8 screws securing the strap to the frame 1.00" thick wood frame Substrate: CMU Block Anchor Capacity (Shear of 3/16" Tapcon)  $P_{ss} / \Omega = 135 \text{ lb}$ (NOA-No. 16-1222.06) Bearing Capacity (of 3/16" Tapcon on strap anchor) P<sub>b</sub> = 2.7 D t F<sub>tu</sub> = 2.7(0.170")(0.033")(45,000 psi) = 681 lb  $P_{allow} = 681 \text{ lb} / 3.0 = 227 \text{ lb}$ Bearing Capacity (of #8 screw on frame)  $P_b = F_e D t / K_D = (3,350 \text{ psi})(0.164'')(1.00'')/(10(0.164) + 0.5) = 257 \text{ lb}$ Bearing Capacity (of #8 screw on strap anchor) P<sub>b</sub> = 2.7 D t F<sub>tu</sub> = 2.7(0.164")(0.033")(45,000 psi) = 657 lb  $P_{allow} = 657 \text{ lb} / 3.0 = 219 \text{ lb}$ Bending Capacity (of 3/16" Tapcon) L = 1/4" (maximum shim space)  $S = \pi d^3 / 32 = \pi (0.170'')^3 / 32 = 0.000482 in^3$ F<sub>b</sub> = (1.3)(0.6 F<sub>y</sub>) = (1.3)(0.6)(137,000 psi) = 106,860 psi (1.3 for weak axis bending)  $F_b = M / S = (V) (L/2) / S$ (L/2 for guided bending) V = 2 S F<sub>b</sub> / L = (2)(0.000482 in3)(106,860 psi) / 1/4" V = 412 lb

Design Capacity of the Connection = 135 lb (one concrete anchor per strap)



## Alternate Installation – Strap Anchor to Wood (Cap Installation)

Anchor: Two #8 x 1-1/2" Flat head screw securing the strap to the substrate

- Details: 20 gauge (0.033" thick) 33 KSI steel strap anchor w/ two #8 screws securing the strap to the frame 1.00" thick wood frame 1/4" max shim space
- Substrate: Spruce-Pine-Fir 2x Wood Substrate (G = 0.42 min.)

<u>Wood Screw Capacity</u> (Withdrawal)

W' = 1.6(82 lb/in)(1.5 in) = <u>197 lb</u>

<u>Pull-over Capacity</u> (of #8 screw on strap)

 $P_{nov} = 1.5 \text{ t d } F_{tu} = 1.5 (0.033")(0.332")(45,000 \text{ psi}) = 739 \text{ lb}$  $P_{allow} = 739 \text{ lb } / 3.0 = 246 \text{ lb}$ 

Bearing Capacity (of #8 screw on frame)

 $P_b = F_e D t / K_D = (3,350 \text{ psi})(0.164")(1.00")/(10(0.164) + 0.5) = 257 \text{ lb}$ 

Bearing Capacity (of #8 screw on strap anchor)

$$\begin{split} P_b &= 2.7 \text{ D t } F_{tu} = 2.7(0.164")(0.033")(45,000 \text{ psi}) = 657 \text{ lb} \\ P_{allow} &= 657 \text{ lb} \ / \ 3.0 = \underline{219 \text{ lb}} \end{split}$$

Design Capacity of the Connection = 197 lb (one screw)

Design Capacity of the Connection = 394 lb (two screws)



#### **Anchorage Requirements**

Series/Model:	Energy Saver Steel Wood Edge Glazed X
Test Unit Size:	38" x 82"
Design Pressure:	+50.0 / -50.0 psf

#### **Through-Frame Installation Method**

Through frame installation method is validated by the test

Through Frame Anchor Capacity = 131 lb

#### Alternate Installation Methods

Strap Anchor to Wood = 244 lb

Through-Frame to Concrete = 181 lb

Through-Frame to CMU Block = 135 lb

Strap Anchor to Concrete = 181 lb

Strap Anchor to CMU Block = 135 lb

Strap Anchor to Wood (Cap Installation) = 197 lb

Minimum Alternate Installation Capacity = 135 lb

135 lb > 131 lb

#### Alternate Anchorages OK at tested spacing



## **Revision Log**

Rev. #	Date	Page(s)	Revision(s)	
0	3/21/2024	All	Original Report Issue	