

Project Name: Energy Saver Steel Wood Edge

Glazed XO

Date: 3/21/2024

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PRODUCT APPROVAL SUPPORTING CALCULATIONS

Series/Model: Energy Saver Steel Wood Edge Glazed XO

REPORT No.: 28227.08-107-16

RENDERED TO: Jeld-Wen Windows & Doors

3737 Lakeport Blvd Klamath Falls, Oregon

PREPARED BY: Michael D. Stremmel, P.E.

Molimo, LLC 1410 Eden Road

York, Pennsylvania 17402

DATE: 3/21/2024

This item has been digitally signed and sealed by Michael D. Stremmel, PE on the date adjacent to the seal.

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Michael D. Stremmel, P.E. Senior Project Engineer FL PE 65868 FL REG 37122

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SCOPE:

Molimo, LLC was contracted by Jeld-Wen Windows & Doors to evaluate alternate installation methods for their Energy Saver Steel Wood Edge Glazed XO. The evaluation is based on physical testing and product certifications.

Reference standards utilized in this project include:

Florida Building Code. International Code Council.

ANSI/AWC National Design Specification (NDS) for Wood Construction. American Wood Council.

AISI S100 North American Specification for the Design of Cold-Formed Steel Structural Members. American Iron and Steel Institute.

ICC-ES Report ESR-1976 ITW Buildex TEKS Self-Drilling Fasteners. ICC Evaluation Service.

NOA 21-0201.06 *Tapcon Concrete and Masonry Anchors with Advanced Threadform Technology*. Miami-Dade County Product Control Section.

The anchorage analysis presented herein does not address the water resistance, water penetration, or air infiltration performance of the installation method or the installed product. In addition, the analyses rely on the assumption that the building substrate is capable of withstanding the incurred loads.



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Certification of Independence

In accordance with Rule 61G20-3 Florida Administrative Code, Molimo, LLC hereby certifies the following:

- Molimo, LLC does not have, nor does it intend to acquire or will it acquire, a financial interest in any company manufacturing or distributing products tested or labeled by the agency.
- Molimo LLC s is not owned, operated or controlled by any company manufacturing or distributing products it tests or labels.
- Michael D. Stremmel, P.E. does not have nor will acquire, a financial interest in any company manufacturing or distributing products for which the reports are being issued.
- Michael D. Stremmel, P.E does not have, nor will acquire, a financial interest in any other entity involved in the approval process of the product.



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ANALYSES:

Summary of Test Results

Table 1 summarizes the various Energy Saver Steel Wood Edge Glazed XO products and their corresponding performance levels which have been established by testing or product certification.

Table 1: Summary of Test Results

Series/Model	Test Report Number	Size (W x H)	Performance
Jeld-Wen Energy Saver Steel	National Certified Test Lab		
Wood Edge	Report No. SJW2010-001	51" x 82"	+50 / -50 psf
Inswing/Outswing Glazed XO	(6/20/2012)		

Testing documented in Table 1 was conducted by National Certified Testing Laboratories of Everett, Washington (Florida Department of Business & Professional Regulation Test Lab No. TST9341 – laboratory was approved at the time of testing). The testing documented above is certified by NAMI under certification number NIO10126.01-R10 (Expires 1/31/2026).

As-Tested Installation Analysis

For air/water/structural testing, the test specimen was secured to a Douglas-Fir wood test buck with #10 wood screws (1-1/2" min. embedment) at the head, sill, and jambs. The as tested installation method is evaluated on Pages 5 and 6. These capacities will be used to prove acceptable anchors and substrates for the product.

Alternate Anchorages

Calculations on Pages 7 through 13 determine the design capacity of alternate installation anchorages for the product.

Anchorages Requirements

As-tested spacing must be maintained. It must be determined that the anchorages are not overloaded for the approved product size and design pressures. Calculations presented on Page 14 show the alternate anchorages are acceptable for the established product performance.

Anchorage requirements established by this report are accurately presented in Drawing D1000382.



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As-Tested Installation - Through Frame to Wood

Anchor: #10 x 2-1/2" Wood Screw

(1-1/2" min embedment})

Details: 0.719" thick wood frame (G = 0.42)

No shim space was utilized

Substrate: Spruce-Pine-Fir wood test buck (G = 0.42)

Wood Screw Capacity (Shear)

Z' = 131 lb (See Following Page)

Design Capacity of the Connection = 131 lb



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Lateral Design Strength of Wood Connections

2.500 in.

Data

<u>Fastener</u>

#10 Wood Screw Fastener Shank Dia 0.190 in. Root Dia. = 0.152 in. 80,000 psi F_{vb} =

Comments:

Project:

Energy Saver Steel Wood Edge Glazed XO

1-1/2" min embedment

Main Member

Fastener length

SPF Material G = 0.42 θ = 90 $F_{\rm e}$ = 3,350 psi Thickness = 1.500 in.

Side Member

Side Memb	<u> </u>		
Material	=	S	PF
G	=	0.42	
θ	=	90	
F_{es}	=	3,350	psi
Thickness	=	0.719	in.

Calculations

Lateral Bearing Factors

D	=	0.152	in
$\ell_{\rm m}$	=	1.451	in
$K_{\boldsymbol{\theta}}$	=	1.25	
K_D	=	2.20	
$R_{\rm e}$	=	1.000	
R_{t}	=	2.02	

\mathbf{k}_1	=	0.6847
k_2	=	1.1271
k_3	=	1.48
R.	=	2.20

$$R_d = 2.20 \quad (Mode I_m, I_s)$$
 $R_d = 2.20 \quad (Mode II)$
 $R_d = 2.20 \quad (Mode III_m, III_s, IV)$

Lateral Design Values, Z

=	336	lbf
=	166	lbf
=	114	lbf
=	126	lbf
=	82	lbf
=	99	lbf
	= = =	= 166 = 114 = 126 = 82

<== Minimum Value

Adjustment Factors

C_D	=	1.6	
V			
Fabrication/In-Service		Dry/Dry	
C_{M}	=	1.0	
In service temperature		T≤100°F	
C_{t}	=	1.0	
C_{σ}	=	1.0	

\mathbf{C}_{Δ} =	1.0
Is fastener installed in end grain?	No
$C_{eg} =$	1.00
Is fastener part of a diaphragm?	No
$C_{di} =$	1.0
Is fastener toe-nailed?	No
C. =	1 00

Adjusted Design Value, Z

131 lbf



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Alternate Installation - Strap Anchor to Wood

Anchor: (2) #10 x 1-1/2" Flat head screws securing the strap to the substrate

1/4" max shim space

Details: 20 gauge (0.033" thick) 33 KSI steel strap anchor w/ two #10 screws securing the

strap to the frame

1-1/2" thick wood frame

Substrate: Spruce-Pine-Fir 2x Wood Substrate (G = 0.42 min.)

Wood Screw Capacity (Shear)

Z' = <u>155 lb</u> (See Following Page)

Bending of #10 x 1-1/2" flat head screw

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.190'')^3 / 32 = 0.000673 in^3$

 $F_b = (1.3)(0.6 F_y) = (1.3)(0.6)(90,000 psi) = 70,200 psi$ (1.3)

(1.3 for weak axis bending)

(L/2 for guided bending)

 $F_b = M / S = (V) (L/2) / S$

 $V = 2 S F_b / L = (2)(0.000221 in 3)(70,200 psi) / 1/4"$

V = 378 lb

Bearing Capacity (of #10 screw on frame)

$$P_b = F_e D t / K_D = (3,350 psi)(0.190")(0.719")/(10(0.190) + 0.5)$$

= <u>191 lb</u> per screw

Bearing Capacity (of strap anchor)

$$P_b = 2.7 D t F_{tu} = 2.7(0.190")(0.033")(45,000 psi) = 761 lb$$

 $P_{allow} = 761 \text{ lb} / 3.0 = 254 \text{ lb} \text{ per screw}$

Design Capacity of the Connection = 155 lb



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Lateral Design Strength of Wood Connections

Data

<u>Fastener</u> #10 Wood Screw Fastener = Shank Dia = 0.190 in. Root Dia. = 0.152 in.

 F_{vb} 80,000 psi =

Fastener length 1.500 in. =

Main Member

Material = SPF G = 0.42 θ = 90 $F_{\rm e}$ = 3,350 psi Thickness

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Comments: Steel Strap Installation 1-1/2" min embedment

= 1.500 in.

Side Member

ASTM A 653, Grade 33 Steel Material G = N/A θ = 90 F_{es} = 61,850 psi Thickness = 0.033 in.

Calculations

Lateral Bearing Factors

D 0.152 in. $\ell_{\rm m}$ = 1.277 in. 1.25 K_{θ} = K_D = 2.20 R_{e} = 0.054 38.70 R_t

0.8550 k_1 k_2 = 0.5357 k_3 26.07 =

 R_d 2.20 (Mode I_m , I_s) = R_d 2.20 (Mode II) = 2.20 (Mode III_m, III_s, IV) R_d

Lateral Design Values, Z

Mode I_m 296 lbf Mode I_s = 141 lbf Mode II 121 = lbf Mode III_m 143 lbf = Mode III_s 97 lbf Mode IV 137 lbf

<== Minimum Value

Adjustment Factors

 C_{D} 1.6 Wet Service Factor Fabrication/In-Service Dry/Dry C_{M} 1.0 T≤100°F In service temperature C_{t} 1.0 1.0 C_{g}

\mathbf{C}_{Δ} =	1.0
Is fastener installed in end grain?	No
$C_{eg} =$	1.00
Is fastener part of a diaphragm?	No
$C_{di} =$	1.0
Is fastener toe-nailed?	No
$C_{tn} =$	1.00

Adjusted Design Value, Z

Z' **155** lbf



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<u>Alternate Installation - Through-Frame to Concrete</u>

Anchor: 3/16" Tapcon Anchor

1-1/4" min embedment
2-1/2" min edge distance
4" min anchor spacing
1/4" max shim space

Details: Through the Wood Frame

- 1" thick

Substrate: 3,000 psi Concrete

Anchor Capacity (Shear of 3/16" Tapcon)

 $P_{ss} / \Omega = 181 \text{ lb}$ (NOA-No. 16-1222.06)

Bearing Capacity (of Wood frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.170")(1.00")/(10(0.170) + 0.5) = 259 lb$

Bending Capacity (of 3/16" Tapcon)

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.170")^3 / 32 = 0.000482 in^3$

 $F_b = (1.3)(0.6 F_v) = (1.3)(0.6)(137,000 psi) = 106,860 psi$ (1.3 for weak axis bending)

 $F_b = M / S = (V) (L/2) / S$ (L/2 for guided bending)

 $V = 2 S F_b / L = (2)(0.000482 in 3)(106,860 psi) / 1/4"$

V = 412 lb

Design Capacity of the Connection = 181 lb



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<u>Alternate Installation – Through-Frame to CMU Block</u>

Anchor: 3/16" Tapcon Anchor

> - 1-1/4" min embedment - 2-1/2" min edge distance - 4" min anchor spacing - 1/4" max shim space

Details: Through the Wood Frame

- 1" thick

Substrate: **CMU Block**

Anchor Capacity (Shear of 3/16" Tapcon)

 $P_{ss}/\Omega = 135 lb$ (NOA-No. 16-1222.06)

Bearing Capacity (of Wood frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.170")(1.00")/(10(0.170) + 0.5) = 259 lb$

Bending Capacity (of 3/16" Tapcon)

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.170'')^3 / 32 = 0.000482 in^3$

 $F_b = (1.3)(0.6 F_v) = (1.3)(0.6)(137,000 psi) = 106,860 psi$ (1.3 for weak axis bending)

 $F_b = M / S = (V) (L/2) / S$ (L/2 for guided bending)

 $V = 2 S F_b / L = (2)(0.000482 in 3)(106,860 psi) / 1/4"$

V = 412 lb

Design Capacity of the Connection = 135 lb



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<u> Alternate Installation – Strap Anchor to Concrete</u>

Anchor: 3/16" Tapcon Anchor

> - 1-1/4" min embedment - 2-1/2" min edge distance - 4" min anchor spacing - 1/4" max shim space

Details: 20 gauge (0.033" thick) 33 KSI steel strap anchor w/ two #8 screws securing the

strap to the frame

1.00" thick wood frame

Substrate: 3,000 psi Concrete

Anchor Capacity (Shear of 3/16" Tapcon)

 $P_{ss}/\Omega = 181 lb$ (NOA-No. 16-1222.06)

Bearing Capacity (of 3/16" Tapcon on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.170")(0.033")(45,000 psi) = 681 lb$

 $P_{allow} = 681 lb / 3.0 = 227 lb$

Bearing Capacity (of #8 screws on frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.164")(1.00")/(10(0.164) + 0.5) = 257 lb$

Bearing Capacity (of #8 screws on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.164")(0.033")(45,000 psi) = 657 lb$

 $P_{allow} = 657 lb / 3.0 = 219 lb$

Bending Capacity (of 3/16" Tapcon)

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.170'')^3 / 32 = 0.000482 in^3$

 $F_b = (1.3)(0.6 F_y) = (1.3)(0.6)(137,000 psi) = 106,860 psi$ (1.3 for weak axis bending)

(L/2 for guided bending)

 $F_b = M / S = (V) (L/2) / S$

 $V = 2 S F_b / L = (2)(0.000482 in 3)(106,860 psi) / 1/4"$

V = 412 lb

Design Capacity of the Connection = 181 lb (one concrete anchor per strap)



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<u>Alternate Installation – Strap Anchor to CMU Block</u>

Anchor: 3/16" Tapcon Anchor

> - 1-1/4" min embedment - 2-1/2" min edge distance - 4" min anchor spacing - 1/4" max shim space

Details: 20 gauge (0.033" thick) 33 KSI steel strap anchor w/ two #8 screws securing the

strap to the frame

1.00" thick wood frame

Substrate: CMU Block

Anchor Capacity (Shear of 3/16" Tapcon)

 $P_{ss}/\Omega = 135 lb$ (NOA-No. 16-1222.06)

Bearing Capacity (of 3/16" Tapcon on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.170")(0.033")(45,000 psi) = 681 lb$

 $P_{allow} = 681 lb / 3.0 = 227 lb$

Bearing Capacity (of #8 screw on frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.164")(1.00")/(10(0.164) + 0.5) = 257 lb$

Bearing Capacity (of #8 screw on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.164")(0.033")(45,000 psi) = 657 lb$

 $P_{allow} = 657 lb / 3.0 = 219 lb$

 $F_b = M / S = (V) (L/2) / S$

Bending Capacity (of 3/16" Tapcon)

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.170'')^3 / 32 = 0.000482 in^3$

 $F_b = (1.3)(0.6 F_y) = (1.3)(0.6)(137,000 psi) = 106,860 psi$ (1.3 for weak axis bending)

(L/2 for guided bending)

 $V = 2 S F_b / L = (2)(0.000482 in 3)(106,860 psi) / 1/4"$

V = 412 lb

Design Capacity of the Connection = 135 lb (one concrete anchor per strap)



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<u>Alternate Installation – Strap Anchor to Wood (Cap Installation)</u>

Anchor: Two #8 x 1-1/2" Flat head screw securing the strap to the substrate

20 gauge (0.033" thick) 33 KSI steel strap anchor w/ two #8 screws securing the Details:

strap to the frame

1.00" thick wood frame 1/4" max shim space

Substrate: Spruce-Pine-Fir 2x Wood Substrate (G = 0.42 min.)

Wood Screw Capacity (Withdrawal)

W' = 1.6(82 lb/in)(1.5 in) = 197 lb

Pull-over Capacity (of #8 screw on strap)

 $P_{nov} = 1.5 \text{ t d } F_{tu} = 1.5 (0.033")(0.332")(45,000 \text{ psi}) = 739 \text{ lb}$

 $P_{allow} = 739 lb / 3.0 = 246 lb$

Bearing Capacity (of #8 screw on frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.164")(1.00")/(10(0.164) + 0.5) = 257 lb$

Bearing Capacity (of #8 screw on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.164")(0.033")(45,000 psi) = 657 lb$

 $P_{allow} = 657 lb / 3.0 = 219 lb$

Design Capacity of the Connection = 197 lb (one screw)

Design Capacity of the Connection = 394 lb (two screws)



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Anchorage Requirements

Series/Model: Energy Saver Steel Wood Edge Glazed XO

51" x 82" Test Unit Size:

Design Pressure: +50.0 / -50.0 psf

Through-Frame Installation Method

Through frame installation method is validated by the test

Through Frame Anchor Capacity = 131 lb

Alternate Installation Methods

Strap Anchor to Wood = 244 lb

Through-Frame to Concrete = 181 lb

Through-Frame to CMU Block = 135 lb

Strap Anchor to Concrete = 181 lb

Strap Anchor to CMU Block = 135 lb

Strap Anchor to Wood (Cap Installation) = 197 lb

Minimum Alternate Installation Capacity = 135 lb

135 lb > 131 lb

Alternate Anchorages OK at tested spacing



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Revision Log

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