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Outswing Opaque High Dam Sill

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PRODUCT APPROVAL SUPPORTING CALCULATIONS

Series/Model Architectural Fiberglass Outswing Opaque High Dam Sill

REPORT No.: 27290.01-107-16

RENDERED TO: Jeld-Wen Windows & Doors

3737 Lakeport Blvd Klamath Falls, Oregon

PREPARED BY: Michael D. Stremmel, P.E.

Molimo, LLC 1410 Eden Road

York, Pennsylvania 17402

DATE: 9/29/2023

This item has been digitally signed and sealed by Michael D. Stremmel, PE on the date adjacent to the seal.

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Michael D. Stremmel, P.E. Senior Project Engineer FL PE 65868 FL REG 37122

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27290.01-107-16 **Architectural Fiberglass**

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SCOPE:

Molimo, LLC was contracted by Jeld-Wen Windows & Doors to evaluate alternate installation methods for their Architectural Fiberglass Outswing Opaque High Dam Sill. The evaluation is based on physical testing and product certifications.

Reference standards utilized in this project include:

Florida Building Code. International Code Council.

ANSI/AWC National Design Specification (NDS) for Wood Construction. American Wood Council.

AISI S100 North American Specification for the Design of Cold-Formed Steel Structural Members. American Iron and Steel Institute.

ICC-ES Report ESR-1976 ITW Buildex TEKS Self-Drilling Fasteners. ICC Evaluation Service.

NOA 21-0201.06 Tapcon Concrete and Masonry Anchors with Advanced Threadform Technology. Miami-Dade County Product Control Section.

The anchorage analysis presented herein does not address the water resistance, water penetration, or air infiltration performance of the installation method or the installed product. In addition, the analyses rely on the assumption that the building substrate is capable of withstanding the incurred loads.



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Certification of Independence

In accordance with Rule 61G20-3 Florida Administrative Code, Molimo, LLC hereby certifies the following:

- Molimo, LLC does not have, nor does it intend to acquire or will it acquire, a financial interest in any company manufacturing or distributing products tested or labeled by the agency.
- Molimo LLC s is not owned, operated or controlled by any company manufacturing or distributing products it tests or labels.
- Michael D. Stremmel, P.E. does not have nor will acquire, a financial interest in any company manufacturing or distributing products for which the reports are being issued.
- Michael D. Stremmel, P.E does not have, nor will acquire, a financial interest in any other entity involved in the approval process of the product.



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ANALYSES:

Summary of Test Results

Table 1 summarizes the various Architectural Fiberglass Outswing Opaque High Dam Sill products and their corresponding performance levels which have been established by testing or product certification.

Table 1: Summary of Test Results

Series/Model	Test Report Number	Size (W x H)	Performance
Architectural Fiberglass	Intertek Report No.	72-3/8" x 80-7/16"	+50 / -50 psf
Outswing Opaque High	H9983.01-301-47-R0		Wind Zone 3
Dam Sill	(Rev. 0, 2/16/18)		Impacts

Testing documented in Table 1 was conducted by Intertek of Fresno, California (Florida Department of Business & Professional Regulation Test Lab No. TST2609, IAS Certificate of Accreditation TL-264).

As-Tested Installation Analysis

For air/water/structural testing, the test specimen was secured to a Douglas-Fir wood test buck with #8 wood screws (1-1/2" min. embedment) through the threshold and 20 ga steel straps at the head and jambs with two #8 wood screws into the frame and two #8 wood screws (1-1/2" min. embedment) through the strap into the wood test buck. The as tested installation method is evaluated on Pages 5 through 9. These capacities will be used to prove acceptable anchors and substrates for the product.

Alternate Anchorages

Calculations on Pages 10 through 16 determine the design capacity of alternate installation anchorages for the product.

Anchorages Requirements

As-tested spacing must be maintained. It must be determined that the anchorages are not overloaded for the approved product size and design pressures. Calculations presented on Page 17 show the alternate anchorages are acceptable for the established product performance.

Anchorage requirements established by this report are accurately presented in Drawing D015317.



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As Tested Installation – Strap Anchor to Wood

Anchor: (2)#8 x 1-1/2" Flat head screw securing the strap to the substrate

No shim space was utilized

Details: 20 gauge (0.036" thick) 33 KSI steel strap anchor w/ two #8 x 1/2" screws

securing the strap to the frame

0.719" thick wood frame

Substrate: Douglas-Fir 2x Wood Substrate (G = 0.42 min.)

Wood Screw Capacity (Shear – Wood Buck Anchor)

Z' = 123 lb (See Page 6)

Wood Screw Capacity (Shear – Frame Anchor)

Z' = 63 lb (See Page 7)

Bearing Capacity (of #8 screw on frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.164")(0.719")/(10(0.164) + 0.5) = 184 lb$

Bearing Capacity (of strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.164")(0.036")(45,000 psi) = 717 lb$

 $P_{allow} = 717 lb / 3.0 = 239 lb$

Design Capacity of the Connection = 63 lb x 2 = 126 lb



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Lateral Design Strength of Wood Connections

Data

<u>Fastener</u> Project: Architectural Fiberglass Outswing

Opaque High Dam Sill

0.036

in.

Fastener #8 Wood Screw **Comments:** Strap Anchor Detail =

1-1/2" min embedment Shank Dia 0.164 in. Root Dia.

 F_{yb} = 90,000 psi

=

=

Main Member Side Member

0.131 in.

1.500 in.

1.500 in.

SPF ASTM A 653, Grade 33 Steel Material Material = G = 0.42 G = N/A θ θ = 90 90 $F_{\rm e}$ 3,350 F_{es} 61,850 = psi psi

Calculations

Thickness

Fastener length

Lateral Bearing Factors

D	=	0.131	in.	\mathbf{k}_1	=	0.7981	
$\ell_{\rm m}$	=	1.300	in.	k_2	=	0.5198	
$K_{\boldsymbol{\theta}}$	=	1.25		k_3	=	21.94	
K_D	=	2.20		R_d	=	2.20	(Mode I _m , I _s)
R_{e}	=	0.054		R_d	=	2.20	(Mode II)
R_{t}	=	36.11		R_d	=	2.20	(Mode III _m , III _s , IV)

Thickness

Lateral Design Values, Z

$ModeI_{m}$	=	259	lbf	
Mode I _s	=	133	lbf	
Mode II	=	106	lbf	
$ModeIII_{m}$	=	122	lbf	
Mode III _s	=	77	lbf	<== Minimum Value
Mode IV	=	108	lbf	

Adjustment Factors

i i a j a b a i i c i i c	1010			
C_D	=	1.6	\mathbf{C}_{Δ} =	1.0
M	Vet Servi	e Factor	Is fastener installed in end grain?	No
Fabrication/In	n-Service	Dry/Dry	C_{eg} =	1.00
C_{M}	=	1.0	Is fastener part of a diaphragm?	No
In service tem	perature	T≤1	$C_{di} = C_{di}$	1.0
C_{t}	=	1.0	Is fastener toe-nailed?	No
C_g	=	1.0	C _{tn} =	1.00

Adjusted Design Value, Z

Z' **123** lbf



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Lateral Design Strength of Wood Connections

#8 Wood Screw

in.

in.

0.164

0.131

90,000 psi

0.500 in.

<u>Data</u>

<u>Fastener</u> Project: Architectural Fiberglass Outswing

Opaque High Dam Sil

Comments: Strap Anchor Detail

Door Frame Anchor

Main Member

 F_{vb}

=

=

=

Fastener

Shank Dia

Root Dia.

Fastener length

SPF Material = 0.42 G = θ = 90 F_{e} = 3,350 psi Thickness 0.719 = in.

Side Member

ASTM A 653, Grade 33 Steel Material G = N/A θ 90 = F_{es} = 61,850 psi Thickness 0.036 in. =

Calculations

Lateral Bearing Factors

D	=	0.131	in.
$\ell_{\rm m}$	=	0.464	in.
$K_{\boldsymbol{\theta}}$	=	1.25	
K_D	=	2.20	
$R_{\rm e}$	=	0.054	
R_{t}	=	12.89	

$$k_1 = 0.2978$$
 $k_2 = 0.9211$
 $k_3 = 21.94$

$$R_{d} = 2.20 \quad (Mode I_{m}, I_{s})$$
 $R_{d} = 2.20 \quad (Mode II)$
 $R_{d} = 2.20 \quad (Mode III_{m}, III_{s}, IV)$

Lateral Design Values, Z

$Mode\ I_{m}$	=	93	lbf
Mode I _s	=	133	lbf
Mode II	=	39	lbf
$ModeIII_{m}$	=	77	lbf
Mode III _s	=	77	lbf
Mode IV	=	108	lbf

<===== Minimum Value

Adjustment Factors

C_D	=	1.6	
W	et Servio	e Factor	
Fabrication/In-	Service	Dry/Dry	
C_{M}	=	1.0	
In service temp	erature	T≤1	100°F
C_{t}	=	1.0	
C_{σ}	=	1.0	

\mathbf{C}_{Δ} =	1.0
Is fastener installed in end grain?	No
$C_{eg} =$	1.00
Is fastener part of a diaphragm?	No
$C_{di} =$	1.0
Is fastener toe-nailed?	No
$C_{tn} =$	1.00

Adjusted Design Value, Z

 $Z' = \underline{63}$ lbf



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As-Tested Installation - Through Threshold to Wood Substrate

Anchor: #8 Wood Screw

Details: 0.719" thick wood frame (G = 0.42)

No shim space was utilized

Substrate: Douglas-Fir wood substrate (G = 0.46)

Wood Screw Capacity (Shear)

Z' = <u>111 lb</u> (See Following Page)

Design Capacity of the Connection = 111 lb



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Lateral Design Strength of Wood Connections

<u>Data</u>

Project: Architectural Fiberglass Outswing

Opaque High Dam Sill **Comments:** As-Tested - Thru-Threhold

1-1/2" min embedment

Main Member

Material	=	Douglas	Fir (South)
G	=	0.46	
θ	=	90	
F_{e}	=	4,000	psi
Thickness	=	1.500	in.

Side Member

Side Mellib	<u>:1</u>		
Material	=	SP	F
G	=	0.42	
θ	=	90	
F_{es}	=	3,350	psi
Thickness	=	0.719	in.

Calculations

Lateral Bearing Factors

D	=	0.131	in.
$\ell_{\rm m}$	=	1.367	in.
$K_{\boldsymbol{\theta}}$	=	1.25	
K_D	=	2.20	
R_{e}	=	1.194	
R_{t}	=	1.90	

\mathbf{k}_1	=	0.7419	
k_2	=	1.2034	
k_3	=	1.29	
R_d	=	2.20	(Mode I_m , I_s)
R_d	=	2.20	(Mode II)
R_d	=	2.20	(Mode III _m , III _s , IV)

Lateral Design Values, Z

$Mode I_{m}$	=	326	lbf
Mode I _s	=	143	lbf
Mode II	=	106	lbf
$Mode III_{m}$	=	116	lbf
Mode III _s	=	69	lbf
Mode IV	=	82	lbf

<== Minimum Value

Adjustment Factors

C_D		=	1.6	
	Wet	t Servic	e Factor	
Fabrication/In-Service			Dry/Dry	
C_{M}		=	1.0	
In service temperature		T≤î	l00°F	
C_t		=	1.0	
C_{σ}		=	1.0	

\mathbf{C}_{Δ} =	1.0
Is fastener installed in end grain?	No
$C_{eg} =$	1.00
Is fastener part of a diaphragm?	No
$C_{di} =$	1.0
Is fastener toe-nailed?	No
$C_{tn} =$	1.00

Adjusted Design Value, Z

Z' = <u>111</u> lbf



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Alternate Installation - Through Frame to Wood

Anchor: #8 Wood Screw

Details: 1-1/4" thick wood frame (G = 0.42)

1/4" max shim space

Substrate: Spruce-Pine-Fir wood substrate (G = 0.42)

Wood Screw Capacity (Shear)

Z' = <u>125 lb</u> (See Following Page)

Bending of #8 x 1-1/2" flat head screw

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.131")^3 / 32 = 0.000221 in^3$

 $F_b = (1.3)(0.6 F_v) = (1.3)(0.6)(90,000 psi) = 70,200 psi$ (1.3 for weak axis bending)

 $F_b = M / S = (V) (L/2) / S$

(L/2 for guided bending)

 $V = 2 S F_b / L = (2)(0.000221 in^3)(70,200 psi) / 1/4"$

V = 124 lb

Design Capacity of the Connection = 125 lb



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Lateral Design Strength of Wood Connections

Data

FastenerFastener=#8 Wood ScrewShank Dia=0.164in.Root Dia.=0.131in. F_{yb} =90,000psiFastener length=2.750in.

Project: Architectural Fiberglass Outswing Opaque High Dam Sill

Comments: Thru-Frame Installation 1-1/2" min embedment

Main Member

Side Member

Side Mellib	<u> </u>			
Material	=	SPF		
G	=	0.42		
θ	=	90		
F_{es}	=	3,350	psi	
Thickness	=	1.250	in.	

Calculations

Lateral Bearing Factors

D	=	0.131	in
$\ell_{\rm m}$	=	1.336	in
$K_{\boldsymbol{\theta}}$	=	1.25	
K_D	=	2.20	
R_{e}	=	1.000	
R_{t}	=	1.07	

\mathbf{k}_1	=	0.4289	
k_2	=	1.1252	
k_3	=	1.14	
R_d	=	2.20	(Mode I_m , I_s)
R_d	=	2.20	(Mode II)
R_d	=	2.20	(Mode III _m , III _s , IV)

Lateral Design Values, Z

Mode I_m 267 lbf Mode Is 249 lbf Mode II 107 lbf 100 Mode III_m lbf Mode III_s 95 lbf Mode IV 78 lbf

<===== Minimum Value

Adjustment Factors

\mathbf{C}_{Δ} =	1.0
Is fastener installed in end grain?	No
$C_{eg} =$	1.00
Is fastener part of a diaphragm?	No
$C_{di} =$	1.0
Is fastener toe-nailed?	No
C. =	1.00

Adjusted Design Value, Z

Z' = <u>125</u> lbf



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<u>Alternate Installation – Through-Frame to Concrete</u>

Anchor: 3/16" Tapcon Anchor

1-1/4" min embedment
2-1/2" min edge distance
3" min anchor spacing
1/4" max shim space

Details: Through the Wood Frame

- 0.719" thick

Substrate: 3,000 psi Concrete

Anchor Capacity (Shear of 3/16" Tapcon)

 $P_{ss} / \Omega = 181 \text{ lb}$ (NOA-No. 21-0201.06)

Bearing Capacity (of Wood frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.170")(0.719")/(10(0.170) + 0.5) = 194 lb$

Bending Capacity (of 3/16" Tapcon)

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.170")^3 / 32 = 0.000482 in^3$

 $F_b = (1.3)(0.6 F_v) = (1.3)(0.6)(137,000 psi) = 106,860 psi$ (1.3 for weak axis bending)

 $F_b = M / S = (V) (L/2) / S$ (L/2 for guided bending)

 $V = 2 S F_b / L = (2)(0.000482 in 3)(106,860 psi) / 1/4"$

V = 412 lb

Design Capacity of the Connection = 181 lb



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<u>Alternate Installation – Through-Frame to CMU Block</u>

Anchor: 3/16" Tapcon Anchor

1-1/4" min embedment
2-1/2" min edge distance
3" min anchor spacing
1/4" max shim space

Details: Through the Wood Frame

- 0.719" thick

Substrate: CMU Block

Anchor Capacity (Shear of 3/16" Tapcon)

 $P_{ss} / \Omega = 135 \text{ lb}$ (NOA-No. 21-0201.06)

Bearing Capacity (of Wood frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.170")(0.719")/(10(0.170) + 0.5) = 194 lb$

Bending Capacity (of 3/16" Tapcon)

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.170")^3 / 32 = 0.000482 in^3$

 $F_b = (1.3)(0.6 F_v) = (1.3)(0.6)(137,000 psi) = 106,860 psi$ (1.3 for weak axis bending)

 $F_b = M / S = (V) (L/2) / S$ (L/2 for guided bending)

 $V = 2 S F_b / L = (2)(0.000482 in3)(106,860 psi) / 1/4"$

V = 412 lb

Design Capacity of the Connection = 135 lb



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<u> Alternate Installation – Strap Anchor to Concrete</u>

Anchor: 3/16" Tapcon Anchor

1-1/4" min embedment
2-1/2" min edge distance
3" min anchor spacing
1/4" max shim space

Details: 20 gauge (0.036" thick) 33 KSI steel strap anchor w/ two #8 screws securing the

strap to the frame

0.719" thick wood frame

Substrate: 3,000 psi Concrete

Anchor Capacity (Shear of 3/16" Tapcon)

 $P_{ss} / \Omega = 181 \text{ lb}$ (NOA-No. 21-0201.06)

Bearing Capacity (of 3/16" Tapcon on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.170")(0.036")(45,000 psi) = 743 lb$

 $P_{allow} = 743 \text{ lb} / 3.0 = 247 \text{ lb}$

Bearing Capacity (of #8 screw on frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.164")(0.719")/(10(0.164) + 0.5) = 184 lb$

Bearing Capacity (of #8 screw on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.164")(0.036")(45,000 psi) = 717 lb$

 $P_{allow} = 717 lb / 3.0 = 239 lb$

Bending Capacity (of 3/16" Tapcon)

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.170'')^3 / 32 = 0.000482 in^3$

 $F_b = (1.3)(0.6 F_y) = (1.3)(0.6)(137,000 psi) = 106,860 psi$

(1.3 for weak axis bending)

 $F_b = M / S = (V) (L/2) / S$ (L/2 for guided bending)

 $V = 2 S F_b / L = (2)(0.000482 in 3)(106,860 psi) / 1/4"$

V = 412 lb

Design Capacity of the Connection = 181 lb (one concrete anchor per strap)



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<u>Alternate Installation – Strap Anchor to CMU Block</u>

Anchor: 3/16" Tapcon Anchor

1-1/4" min embedment
2-1/2" min edge distance
3" min anchor spacing
1/4" max shim space

Details: 20 gauge (0.036" thick) 33 KSI steel strap anchor w/ two #8 screws securing the

strap to the frame

0.719" thick wood frame

Substrate: CMU Block

Anchor Capacity (Shear of 3/16" Tapcon)

 $P_{ss} / \Omega = 135 lb$ (NOA-No. 21-0201.06)

Bearing Capacity (of 3/16" Tapcon on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.170")(0.036")(45,000 psi) = 743 lb$

 $P_{allow} = 743 \text{ lb} / 3.0 = 247 \text{ lb}$

Bearing Capacity (of #8 screw on frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.164")(0.719")/(10(0.164) + 0.5) = 184 lb$

Bearing Capacity (of #8 screw on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.164")(0.036")(45,000 psi) = 717 lb$

 $P_{allow} = 717 lb / 3.0 = 239 lb$

Bending Capacity (of 3/16" Tapcon)

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.170'')^3 / 32 = 0.000482 in^3$

 $F_b = (1.3)(0.6 F_y) = (1.3)(0.6)(137,000 psi) = 106,860 psi$

(1.3 for weak axis bending)

 $F_b = M / S = (V) (L/2) / S$ (L/2 for guided bending)

 $V = 2 S F_b / L = (2)(0.000482 in 3)(106,860 psi) / 1/4"$

V = 412 lb

Design Capacity of the Connection = 135 lb (one concrete anchor per strap)



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Anchor: One #8 x 1-1/2" Flat head screw securing the strap to the substrate

Details: 20 gauge (0.036" thick) 33 KSI steel strap anchor w/ two #8 screws securing the

strap to the frame

0.719" thick wood frame 1/4" max shim space

Substrate: Spruce-Pine-Fir 2x Wood Substrate (G = 0.42 min.)

Alternate Installation - Strap Anchor to Wood (Cap Installation)

Wood Screw Capacity (Withdrawal)

W' = 1.6(82 lb/in)(1.5 in) = 197 lb

Pull-over Capacity (of #8 screw on strap)

 $P_{nov} = 1.5 \text{ t d } F_{tu} = 1.5 (0.036")(0.332")(45,000 \text{ psi}) = 806 \text{ lb}$

 $P_{allow} = 806 \text{ lb} / 3.0 = 268 \text{ lb}$

Bearing Capacity (of #8 screw on frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.164")(0.719")/(10(0.164) + 0.5) = 184 lb$

Bearing Capacity (of #8 screw on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.164")(0.036")(45,000 psi) = 717 lb$

 $P_{allow} = 717 lb / 3.0 = 239 lb$

Design Capacity of the Connection = 184 lb (one screw)



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Anchorage Requirements

Series/Model: Architectural Fiberglass Outswing Opaque High Dam Sill

Test Unit Size: 72-3/8" x 80-7/16"

Design Pressure: +50.0 / -50.0 psf

Anchor Strap to Wood Substrate Installation Method (As Tested)

Anchor Strap installation method to wood substrate and Threshold anchor to wood substrate is validated by the test wit

Through Threshold Anchor Capacity = 111 lb / anchor

Anchor Strap Capacity = 126 lb / anchor

Alternate Installation Methods

Thru-Frame Anchor to Wood = 125 lb / anchor

Through-Frame to Concrete = 181 lb / anchor

Through-Frame to CMU Block = 135 lb / anchor

Strap Anchor to Concrete = 181 lb / anchor

Strap Anchor to CMU Block = 135 lb / anchor

Strap Anchor to Wood (Cap Installation) = 184 lb / anchor

Minimum Alternate Installation Capacity = 125 lb / anchor

125 lb ~ 126 lb (< 1% difference OK)

Alternate Anchorages OK at tested spacing



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