

Date:

Project Name: Architectural Fiberglass Inswing

Opaque Double Door 9/29/2023

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PRODUCT APPROVAL SUPPORTING CALCULATIONS

Series/Model Architectural Fiberglass Inswing Opaque Double Door

REPORT No.: 27291.01-107-16

RENDERED TO: Jeld-Wen Windows & Doors

3737 Lakeport Blvd Klamath Falls, Oregon

PREPARED BY: Michael D. Stremmel, P.E.

Molimo, LLC 1410 Eden Road

York, Pennsylvania 17402

DATE: 9/29/2023

This item has been digitally signed and sealed by Michael D. Stremmel, PE on the date adjacent to the seal.

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Michael D. Stremmel, P.E. Senior Project Engineer FL PE 65868 FL REG 37122

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SCOPE:

Molimo, LLC was contracted by Jeld-Wen Windows & Doors to evaluate alternate installation methods for their Architectural Fiberglass Inswing Opaque Double Door. The evaluation is based on physical testing and product certifications.

Reference standards utilized in this project include:

Florida Building Code. International Code Council.

ANSI/AWC *National Design Specification (NDS) for Wood Construction*. American Wood Council.

AISI S100 North American Specification for the Design of Cold-Formed Steel Structural Members. American Iron and Steel Institute.

ICC-ES Report ESR-1976 ITW Buildex TEKS Self-Drilling Fasteners. ICC Evaluation Service.

NOA 21-0201.06 Tapcon Concrete and Masonry Anchors with Advanced Threadform Technology. Miami-Dade County Product Control Section.

The anchorage analysis presented herein does not address the water resistance, water penetration, or air infiltration performance of the installation method or the installed product. In addition, the analyses rely on the assumption that the building substrate is capable of withstanding the incurred loads.



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Certification of Independence

In accordance with Rule 61G20-3 Florida Administrative Code, Molimo, LLC hereby certifies the following:

- Molimo, LLC does not have, nor does it intend to acquire or will it acquire, a financial interest in any company manufacturing or distributing products tested or labeled by the agency.
- Molimo LLC s is not owned, operated or controlled by any company manufacturing or distributing products it tests or labels.
- Michael D. Stremmel, P.E. does not have nor will acquire, a financial interest in any company manufacturing or distributing products for which the reports are being issued.
- Michael D. Stremmel, P.E does not have, nor will acquire, a financial interest in any other entity involved in the approval process of the product.



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ANALYSES:

Summary of Test Results

Table 1 summarizes the various Architectural Fiberglass Inswing Opaque Double Door products and their corresponding performance levels which have been established by testing or product certification.

Table 1: Summary of Test Results

Series/Model	Test Report Number	Size (W x H)	Performance
Architectural Fiberglass Inswing Opaque Double Door	National Certified Testing Laboratories Report No. NCTL-310-19-031 E1A0 (Rev. 1, 3/04/20)	73-3/4" x 81-7/8"	+60 / -60 psf Wind Zone 3 Impacts

Testing documented in Table 1 was conducted by National Certified Testing Laboratories of Everett, Washington (Florida Department of Business & Professional Regulation Test Lab No. TST9341, A2LA Certificate of Accreditation 3054.03).

As-Tested Installation Analysis

For air/water/structural testing, the test specimen was secured to a Spruce-Pine-Fir wood test buck with #8 wood screws (1-1/2" min. embedment) at the head and jambs and a continuous bed of silicone at the sill. The as tested installation method is evaluated on Pages 5 and 6. These capacities will be used to prove acceptable anchors and substrates for the product.

Alternate Anchorages

Calculations on Pages 7 through 13 determine the design capacity of alternate installation anchorages for the product.

Anchorages Requirements

As-tested spacing must be maintained. It must be determined that the anchorages are not overloaded for the approved product size and design pressures. Calculations presented on Page 14 show the alternate anchorages are acceptable for the established product performance.

Anchorage requirements established by this report are accurately presented in Drawing D015591.



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<u>As-Tested Installation – Through Frame to Wood</u>

Anchor: #8 x 2-1/2" Wood Screw (1-1/2" min embedment)

Details: 0.719" thick wood frame (G = 0.42)

No shim space was utilized

Substrate: Spruce-Pine-Fir wood test buck (G = 0.42)

Wood Screw Capacity (Shear)

Z' = <u>107 lb</u> (See Following Page)

Design Capacity of the Connection = 107 lb



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Lateral Design Strength of Wood Connections

<u>Data</u>

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Comments: As-Tested

1-1/2" min embedment

Main Member

Material	=	S	SPF
G	=	0.42	
θ	=	90	
F_{e}	=	3,350	psi
Thickness	=	1.500	in.

Side Member

 R_{d}

01440114	<u> </u>		
Material	=	SI	PF
G	=	0.42	
θ	=	90	
F_{es}	=	3,350	psi
Thickness	=	0.719	in.

Calculations

Lateral Bearing Factors

D	=	0.131	in.
$\ell_{\rm m}$	=	1.203	in.
$K_{\boldsymbol{\theta}}$	=	1.25	
K_D	=	2.20	
R_{e}	=	1.000	
R_{t}	=	1.67	

k_1	=	0.5834	
k_2	=	1.1534	
k_3	=	1.40	
R_d	=	2.20	(Mode I _m , I _s)
R_d	=	2.20	(Mode II)

2.20

(Mode III_m, III_s, IV)

Lateral Design Values, Z

$\operatorname{Mode} I_{m}$	=	240	lbf
Mode I _s	=	143	lbf
Mode II	=	84	lbf
$ModeIII_{m}$	=	92	lbf
Mode III _s	=	67	lbf
Mode IV	=	78	lbf

<== Minimum Value

Adjustment Factors

$C_D =$	1.6	
Wet Servio	e Factor	
Fabrication/In-Service	Dry/Dry	
$C_{M} =$	1.0	
In service temperature	T≤1	100°F
$C_t =$	1.0	
$C_g =$	1.0	

\mathbf{C}_{Δ} =	1.0
Is fastener installed in end grain?	No
$C_{eg} =$	1.00
Is fastener part of a diaphragm?	No
$C_{di} =$	1.0
Is fastener toe-nailed?	No
$C_{tn} =$	1.00

Adjusted Design Value, Z

Z' = <u>107</u> lbf



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Alternate Installation - Strap Anchor to Wood

Anchor: (2)#8 x 1-1/2" Flat head screw securing the strap to the substrate

1/4" max shim space

Details: 20 gauge (0.036" thick) 33 KSI steel strap anchor w/ two #8 screws securing the

strap to the frame

0.719" thick wood frame

Substrate: Spruce-Pine-Fir 2x Wood Substrate (G = 0.42 min.)

Wood Screw Capacity (Shear)

Z' = 122 lb (See Following Page)

Bending of #8 x 1-1/2" flat head screw

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.131'')^3 / 32 = 0.000221 in^3$

 $F_b = (1.3)(0.6 F_y) = (1.3)(0.6)(90,000 psi) = 70,200 psi$

(1.3 for weak axis bending)(L/2 for guided bending)

 $F_b = M / S = (V) (L/2) / S$

 $V = 2 S F_b / L = (2)(0.000221 in3)(70,200 psi) / 1/4"$

V = 124 lb

Bearing Capacity (of #8 screw on frame)

$$P_b = F_e D t / K_D = (3,350 psi)(0.164")(0.719")/(10(0.164) + 0.5) = 184 lb$$

Bearing Capacity (of strap anchor)

$$P_b = 2.7 D t F_{tu} = 2.7(0.164")(0.036")(45,000 psi) = 717 lb$$

 $P_{allow} = 717 lb / 3.0 = 239 lb$

Design Capacity of the Connection = 122 lb x 2 = 244 lb



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Lateral Design Strength of Wood Connections

Data

Fastener = #8 Wood Screw

Shank Dia = 0.164 in. Root Dia. = 0.131 in.

 F_{yb} = 90,000 psi Fastener length = 1.500 in. **Project:** Architectural Fiberglass Inswing

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Comments: Strap Anchor Detail

1-1/2" min embedment

Main Member

Material	=	5	SPF
G	=	0.42	
θ	=	90	
F_{e}	=	3,350	psi
Thickness	=	1.500	in.

Side Member

Dide Mellio	CI		
Material	=	ASTM A 653, 0	Frade 33 Steel
G	=	N/A	
θ	=	90	
F_{es}	=	61,850	psi
Thickness	=	0.033	in.

Calculations

Lateral Bearing Factors

D	=	0.131	in.
$\ell_{\rm m}$	=	1.139	in.
$K_{\boldsymbol{\theta}}$	=	1.25	
K_D	=	2.20	
R_{e}	=	0.054	
R_{t}	=	34.52	

\mathbf{k}_1	=	0.7630	
k_2	=	0.5398	
k_3	=	23.87	
R_d	=	2.20	(Mode I_m , I_s)
R_d	=	2.20	(Mode II)
R_d	=	2.20	(Mode III _m , III _s , IV)

Lateral Design Values, Z

$ModeI_{m}$	=	227	lbf
Mode I _s	=	122	lbf
Mode II	=	93	lbf
$ModeIII_{m}$	=	111	lbf
Mode III _s	=	77	lbf
Mode IV	=	108	lbf

<== Minimum Value

Adjustment Factors

	C_D	=	1.6		
Wet Service Factor					
Fabrication/In-Service		Dry/Dry			
	C_{M}	=	1.0		
In service temperature		T≤100°F			
	C_{t}	=	1.0		
	C_g	=	1.0		

$C_{\!\vartriangle}$ =	1.0
Is fastener installed in end grain?	No
$C_{eg} =$	1.00
Is fastener part of a diaphragm?	No
$C_{di} =$	1.0
Is fastener toe-nailed?	No
$C_{tn} =$	1.00

Adjusted Design Value, Z

 $Z' = \frac{122}{\text{lbf}}$



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<u>Alternate Installation - Through-Frame to Concrete</u>

Anchor: 3/16" Tapcon Anchor

1-1/4" min embedment
2-1/2" min edge distance
3" min anchor spacing
1/4" max shim space

Details: Through the Wood Frame

- 0.719" thick

Substrate: 3,000 psi Concrete

Anchor Capacity (Shear of 3/16" Tapcon)

 $P_{ss} / \Omega = 181 \text{ lb}$ (NOA-No. 21-0201.06)

Bearing Capacity (of Wood frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.170")(0.719")/(10(0.170) + 0.5) = 194 lb$

Bending Capacity (of 3/16" Tapcon)

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.170")^3 / 32 = 0.000482 in^3$

 $F_b = (1.3)(0.6 F_v) = (1.3)(0.6)(137,000 psi) = 106,860 psi$ (1.3 for weak axis bending)

 $F_b = M / S = (V) (L/2) / S$ (L/2 for guided bending)

 $V = 2 S F_b / L = (2)(0.000482 in 3)(106,860 psi) / 1/4"$

V = 412 lb

Design Capacity of the Connection = 181 lb



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<u>Alternate Installation – Through-Frame to CMU Block</u>

Anchor: 3/16" Tapcon Anchor

1-1/4" min embedment
2-1/2" min edge distance
3" min anchor spacing
1/4" max shim space

Details: Through the Wood Frame

- 0.719" thick

Substrate: CMU Block

Anchor Capacity (Shear of 3/16" Tapcon)

 $P_{ss} / \Omega = 135 lb$ (NOA-No. 21-0201.06)

Bearing Capacity (of Wood frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.170")(0.719")/(10(0.170) + 0.5) = 194 lb$

Bending Capacity (of 3/16" Tapcon)

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.170")^3 / 32 = 0.000482 in^3$

 $F_b = (1.3)(0.6 F_v) = (1.3)(0.6)(137,000 psi) = 106,860 psi$ (1.3 for weak axis bending)

 $F_b = M / S = (V) (L/2) / S$ (L/2 for guided bending)

 $V = 2 S F_b / L = (2)(0.000482 in 3)(106,860 psi) / 1/4"$

V = 412 lb

Design Capacity of the Connection = 135 lb



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<u>Alternate Installation – Strap Anchor to Concrete</u>

Anchor: 3/16" Tapcon Anchor

1-1/4" min embedment
2-1/2" min edge distance
3" min anchor spacing
1/4" max shim space

Details: 20 gauge (0.036" thick) 33 KSI steel strap anchor w/ two #8 screws securing the

strap to the frame

0.719" thick wood frame

Substrate: 3,000 psi Concrete

Anchor Capacity (Shear of 3/16" Tapcon)

 $P_{ss} / \Omega = 181 \text{ lb}$ (NOA-No. 21-0201.06)

Bearing Capacity (of 3/16" Tapcon on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.170")(0.036")(45,000 psi) = 743 lb$

 $P_{allow} = 743 \text{ lb} / 3.0 = 247 \text{ lb}$

Bearing Capacity (of #8 screw on frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.164")(0.719")/(10(0.164) + 0.5) = 184 lb$

Bearing Capacity (of #8 screw on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.164")(0.036")(45,000 psi) = 717 lb$

 $P_{allow} = 717 lb / 3.0 = 239 lb$

Bending Capacity (of 3/16" Tapcon)

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.170'')^3 / 32 = 0.000482 in^3$

 $F_b = (1.3)(0.6 F_y) = (1.3)(0.6)(137,000 psi) = 106,860 psi$

(1.3 for weak axis bending)

 $F_b = M / S = (V) (L/2) / S$

(L/2 for guided bending)

 $V = 2 S F_b / L = (2)(0.000482 in3)(106,860 psi) / 1/4"$

V = 412 lb

Design Capacity of the Connection = 181 lb (one concrete anchor per strap)



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Alternate Installation - Strap Anchor to CMU Block

Anchor: 3/16" Tapcon Anchor

1-1/4" min embedment
2-1/2" min edge distance
3" min anchor spacing
1/4" max shim space

Details: 20 gauge (0.036" thick) 33 KSI steel strap anchor w/ two #8 screws securing the

strap to the frame

0.719" thick wood frame

Substrate: CMU Block

Anchor Capacity (Shear of 3/16" Tapcon)

 $P_{ss} / \Omega = 135 \text{ lb}$ (NOA-No. 21-0201.06)

Bearing Capacity (of 3/16" Tapcon on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.170")(0.036")(45,000 psi) = 743 lb$

 $P_{allow} = 743 \text{ lb} / 3.0 = 247 \text{ lb}$

Bearing Capacity (of #8 screw on frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.164")(0.719")/(10(0.164) + 0.5) = 184 lb$

Bearing Capacity (of #8 screw on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.164")(0.036")(45,000 psi) = 717 lb$

 $P_{allow} = 717 lb / 3.0 = 239 lb$

Bending Capacity (of 3/16" Tapcon)

L = 1/4" (maximum shim space)

 $S = \pi d^3 / 32 = \pi (0.170'')^3 / 32 = 0.000482 in^3$

 $F_b = (1.3)(0.6 F_y) = (1.3)(0.6)(137,000 psi) = 106,860 psi$

(1.3 for weak axis bending)

 $F_b = M / S = (V) (L/2) / S$ (L/2 for guided bending)

 $V = 2 S F_b / L = (2)(0.000482 in 3)(106,860 psi) / 1/4"$

V = 412 lb

Design Capacity of the Connection = 135 lb (one concrete anchor per strap)



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<u>Alternate Installation – Strap Anchor to Wood (Cap Installation)</u>

Anchor: Two #8 x 1-1/2" Flat head screw securing the strap to the substrate

Details: 20 gauge (0.036" thick) 33 KSI steel strap anchor w/ two #8 screws securing the

strap to the frame

0.719" thick wood frame 1/4" max shim space

Substrate: Spruce-Pine-Fir 2x Wood Substrate (G = 0.42 min.)

Wood Screw Capacity (Withdrawal)

W' = 1.6(82 lb/in)(1.5 in) = 197 lb

Pull-over Capacity (of #8 screw on strap)

 $P_{nov} = 1.5 \text{ t d } F_{tu} = 1.5 (0.036")(0.332")(45,000 \text{ psi}) = 806 \text{ lb}$

 $P_{allow} = 806 \text{ lb} / 3.0 = 268 \text{ lb}$

Bearing Capacity (of #8 screw on frame)

 $P_b = F_e D t / K_D = (3,350 psi)(0.164")(0.719")/(10(0.164) + 0.5) = 184 lb$

Bearing Capacity (of #8 screw on strap anchor)

 $P_b = 2.7 D t F_{tu} = 2.7(0.164")(0.036")(45,000 psi) = 717 lb$

 $P_{allow} = 717 lb / 3.0 = 239 lb$

Design Capacity of the Connection = 184 lb (one screw)

Design Capacity of the Connection = 368 lb (two screws)



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Anchorage Requirements

Series/Model: Architectural Fiberglass Inswing Opaque Double Door

Test Unit Size: 73-3/4" x 81-7/8"

Design Pressure: +60.0 / -60.0 psf

Through-Frame Installation Method

Through frame installation method is validated by the test

Through Frame Anchor Capacity = 107 lb / anchor

Alternate Installation Methods

Strap Anchor to Wood = 122 lb / anchor

Through-Frame to Concrete = 181 lb / anchor

Through-Frame to CMU Block = 135 lb / anchor

Strap Anchor to Concrete = 181 lb / anchor

Strap Anchor to CMU Block = 135 lb / anchor

Strap Anchor to Wood (Cap Installation) = 184 lb / anchor

Minimum Alternate Installation Capacity = 122 lb / anchor

122 lb > 107 lb

Alternate Anchorages OK at tested spacing



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Revision Log

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