

**ICC-ES Evaluation Report****ESR-2427\***

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**DIVISION: 03 00 00—CONCRETE**  
**Section: 03 16 00—Concrete Anchors****DIVISION: 05 00 00—METALS**  
**Section: 05 05 19—Post-installed Concrete Anchors****REPORT HOLDER:****ITW RED HEAD**  
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[www.itwbrands.com](http://www.itwbrands.com)**EVALUATION SUBJECT:****ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHORS, STAINLESS STEEL TRUBOLT+ WEDGE ANCHORS AND CARBON STEEL OVERHEAD TRUBOLT+ WEDGE ANCHORS FOR CRACKED AND UNCRACKED CONCRETE****1.0 EVALUATION SCOPE****Compliance with the following codes:**

- 2012, 2009, and 2006 *International Building Code*® (IBC)
- 2012, 2009, and 2006 *International Residential Code*® (IRC)

**Property evaluated:**

Structural

**2.0 USES**

The carbon steel and stainless steel Trubolt+ Wedge Anchors and  $\frac{3}{8}$ -inch-diameter (9.5 mm) carbon steel OVERHEAD Trubolt+ Wedge Anchor are used to resist static, wind, and seismic tension and shear loads in cracked and uncracked normal-weight and sand-lightweight concrete having a specified compressive strength,  $f'_c$ , ranging from 2,500 psi to 8,500 psi (17.2 MPa to 58.6 MPa).

The carbon steel Trubolt+ Wedge Anchors with diameters of  $\frac{3}{8}$  inch (9.5 mm),  $\frac{1}{2}$  inch (12.7 mm) and  $\frac{5}{8}$ -inch (15.9 mm) and the carbon steel OVERHEAD  $\frac{3}{8}$ -inch-diameter (9.5 mm) are used to resist static, wind, and seismic tension and shear loads in cracked and uncracked normal-weight or sand-lightweight concrete over steel deck having a minimum specified compressive strength,  $f'_c$ , of 3,000 psi (20.7 MPa).

The Trubolt+ Wedge anchors comply with anchors as described in Section 1909 of the 2012 IBC, and Section 1912 of the 2009 and 2006 IBC. The anchors are alternatives to cast-in-place anchors described in Section 1908 of the 2012 IBC, and Section 1911 of the 2009 and 2006 IBC. The anchors may also be used where an engineered design is submitted in accordance with Section R301.1.3 of the IRC.

**3.0 DESCRIPTION****3.1 RED HEAD Carbon Steel Trubolt+ Wedge Anchor:**

The RED HEAD Trubolt+ Wedge Anchor is a torque-controlled, wedge-type mechanical expansion anchor, available in  $\frac{3}{8}$ -inch (9.5 mm),  $\frac{1}{2}$ -inch (12.7 mm),  $\frac{5}{8}$ -inch (15.9 mm) and  $\frac{3}{4}$ -inch (19.1 mm) diameters. The Trubolt+ Wedge Anchor consists of a high-strength threaded anchor body, expansion clip, hex nut and washer. The anchor body is cold-formed from low carbon steel materials conforming to AISI 1015 or AISI 1018 with mechanical properties (yield and ultimate strengths) as described in Tables 3 and 4 of this report. The zinc plating on the anchor body complies with ASTM B633 SC1, Type III, with a minimum 0.0002-inch (5  $\mu$ m) thickness. The expansion clip is fabricated from low carbon steel materials conforming to AISI 1020. The standard hexagonal steel nut conforms to ANSI B18.2.2-65 and the washer conforms to ANSI/ASME B18.22.1 1965 (R1981). The Trubolt+ Wedge anchor body consists of a threaded section throughout the majority of its length and a wedge section at the far end. The expansion clip is formed around the anchor, just above the wedge. The expansion clip consists of a split cylindrical ring with undercutting grooves at the bottom end. During torquing of the anchor, the grooves in the expansion clip are designed to cut into the walls of the concrete hole as the wedge portion of the stud is forced upward against the interior of the clip (U.S. patent nos. 7,744,320 and 7,811,037). The Trubolt+ Wedge anchor is illustrated in Figure 1 of this report.

**3.2 RED HEAD Stainless Steel Trubolt+ Wedge Anchor:**

The RED HEAD Trubolt+ Wedge Anchor is a torque-controlled, wedge-type mechanical expansion anchor,

**\*Revised May 2014**

available in  $1/2$ -inch (12.7 mm) and  $5/8$ -inch (15.9 mm) diameters. The Trubolt+ Wedge Anchor consists of a high-strength threaded anchor body, expansion clip, hex nut and washer. The anchor body is cold-formed from AISI Type 316 stainless steel materials with mechanical properties (yield and ultimate strengths) as described in Tables 5 and 6 of this report. The expansion clip is fabricated from Type 316 stainless steel materials. The Type 316 stainless steel hexagonal steel nut conforms to ANSI B18.2.2-65 and the AISI Type 316 stainless steel washer conforms to ANSI/ASME B18.22.1 1965 (R1981). The Trubolt+ Wedge anchor body consists of a threaded section throughout the majority of its length and a wedge section at the far end. The expansion clip is formed around the anchor, just above the wedge. The expansion clip consists of a split cylindrical ring with undercutting grooves at the bottom end. During torquing of the anchor, the grooves in the expansion clip are designed to cut into the walls of the concrete hole as the wedge portion of the stud is forced upward against the interior of the clip. The Trubolt+ Wedge anchor is illustrated in Figure 1 of this report.

### 3.3 OVERHEAD Trubolt+ Wedge Anchor:

The OVERHEAD Trubolt+ Wedge Anchor is a torque-controlled, wedge-type mechanical expansion anchor, available in  $3/8$ -inch (9.5 mm) diameter. The OVERHEAD Trubolt+ Wedge Anchor consists of a high-strength threaded anchor body, expansion clip, coupling nut and washer. The anchor body is cold-formed from low carbon steel materials with the mechanical properties (yield and ultimate strengths) as described in Tables 3 and 4 of this report. The zinc plating on the anchor body complies with ASTM B633 SC1, Type III, with a minimum 0.0002 inch (5  $\mu$ m) thickness. The expansion clip is fabricated from low carbon steel materials. The coupling nut consists of Grade 2 steel with  $3/8$ " -16 threads throughout the length of the nut. The washer complies with ANSI/ASME B18.22.1 1965 (R1981). The OVERHEAD Trubolt+ Wedge anchor body consists of a threaded section throughout the majority of its length and a wedge section at the far end. The expansion clip is formed around the anchor, just above the wedge. The expansion clip consists of a split cylindrical ring with undercutting grooves at the bottom end. During torquing of the anchor (using coupling nut), the grooves in the expansion clip are designed to cut into the walls of the concrete hole as the wedge portion of the anchor body is forced upward against the interior of the clip (U.S. patent nos. 7,744,320 and 7,811,037). The OVERHEAD Trubolt+ Wedge anchor is illustrated in Figure 2 of this report.

### 3.4 Concrete:

Normal-weight and sand-lightweight concrete must comply with Sections 1903 and 1905 of the IBC.

### 3.5 Steel Deck Panels:

Steel deck panels must comply with ASTM A653, SS Grade 40 (minimum), and must have a minimum 0.034-inch (0.864 mm) base-metal thickness (No. 20 gage).

## 4.0 DESIGN AND INSTALLATION

### 4.1 Strength Design:

**4.1.1 General:** Design strength of anchors in accordance with the 2012 IBC, as well as Section R301.1.3 of the 2012 IRC, must be determined in accordance with ACI 318-11 Appendix D and this report.

Design strength of anchors in accordance with the 2009 IBC and Section R301.1.3 of the 2009 IRC must be in accordance with ACI 318-08 Appendix D and this report.

Design strength of anchors in accordance with the 2006 IBC and Section R301.1.3 of the 2006 IRC must be in accordance with ACI 318-05 Appendix D and this report.

Design parameters are based on the 2012 IBC (ACI 318-11) unless noted otherwise in Sections 4.1.1 through 4.1.12 of this report. The strength design of anchors must comply with ACI 318 D.4.1, except as required in ACI 318 D.3.3. Strength reduction factors,  $\phi$ , as given in ACI 318-11 D.4.3 (ACI 318-08 and -05 D.4.4) must be used for load combinations calculated in accordance with Section 1605.2 of the IBC and Section 9.2 of ACI 318. Strength reduction factors,  $\phi$ , as given in ACI 318-11 D.4.4 (ACI 318-08 and -05 D.4.5) must be used for load combinations calculated in accordance with ACI 318 Appendix C. The value of  $f'_c$  used in calculations must be limited to 8,000 psi (55.2 MPa), maximum, in accordance with ACI 318-11 D.3.7. Strength reduction factors,  $\phi$ , corresponding to ductile steel elements may be used except for the carbon steel  $3/8$ -inch-diameter (9.5 mm) anchors loaded in shear, which have a strength reduction factor corresponding to brittle steel elements.

**4.1.2 Requirements for Static Steel Strength in Tension,  $N_{sa}$ :** The nominal static steel strength of a single anchor in tension,  $N_{sa}$ , calculated in accordance with ACI 318, Section D.5.1.2, is given in Tables 3 or 5 of this report.

**4.1.3 Requirements for Static Concrete Breakout Strength in Tension,  $N_{cb}$ ,  $N_{cbg}$ :** The nominal concrete breakout strength of a single anchor or a group of anchors in tension,  $N_{cb}$  or  $N_{cbg}$  respectively, must be calculated in accordance with ACI 318 D.5.2, with modifications as described in this section. The values of  $f'_c$  used for calculation purposes must not exceed 8,000 psi (55.0 MPa). The basic concrete breakout strength of a single anchor in tension,  $N_b$ , must be calculated in accordance with ACI 318 D.5.2.2, using the values of  $h_{ef}$  and  $k_{cr}$  as given in Tables 3 or 5 of this report. The nominal concrete breakout strength in tension in regions where analysis indicates no cracking in accordance with ACI 318 D.5.2.6 must be calculated with  $\psi_{c,N} = 1.0$  and using the value of  $k_{uncr}$  as given in Tables 3 or 5 of this report.

For anchors installed in the soffit of sand-lightweight or normal-weight concrete-filled steel deck floor and roof assemblies, as shown in Figure 7, calculation of the concrete breakout strength in accordance with ACI 318 D.5.2 is not required.

**4.1.4 Requirements for Static Pullout Strength in Tension,  $N_{pn}$ :** The nominal pullout strength of a single anchor in tension in accordance with ACI 318 D.5.3 in cracked and uncracked concrete,  $N_{p,cr}$  or  $N_{p,uncr}$ , respectively, is given in Tables 3 or 5 of this report. For all design cases,  $\psi_{c,P} = 1.0$ . In accordance with ACI 318 D.5.3.2, the nominal pullout strength in tension must be calculated according to Eq-1.

$$N_{p,f'c} = N_{p,cr} \sqrt{\frac{f'_c}{2,500}} \quad (\text{lb, psi}) \quad (\text{Eq-1})$$

$$N_{p,f'c} = N_{p,cr} \sqrt{\frac{f'_c}{17.2}} \quad (\text{N, MPa})$$

In regions where analysis indicates no cracking in accordance with ACI 318 D.5.3.6, the nominal pullout strength in tension must be calculated according to Eq-2:

$$N_{p,f'c} = N_{p,uncr} \sqrt{\frac{f'_c}{2,500}} \quad (\text{lb, psi}) \quad (\text{Eq-2})$$

$$N_{p,f_c} = N_{p,uncr} \sqrt{\frac{f_c}{17.2}} \quad (\text{N, MPa})$$

where values for  $N_{p,cr}$  or  $N_{p,uncr}$  are not provided in Tables 3 or 5 of this report, the pullout strength in tension need not be evaluated.

The nominal pullout strength in tension of the anchors installed in the soffit of sand lightweight or normal-weight concrete on steel deck floor and roof assemblies, as shown in Figure 7 of this report, is given in Table 9. In accordance with ACI 318 D.5.3.2, the nominal pullout strength in cracked concrete must be calculated according to Eq-1, whereby the value of  $N_{p,deck,cr}$  must be substituted for  $N_{p,cr}$  and the value 3,000 psi or 20.7 MPa must be substituted for 2,500 psi or 17.2 MPa. In regions where analysis indicates no cracking in accordance with ACI 318 D.5.3.6, the nominal pullout strength in tension must be calculated according to Eq-2, whereby the value of  $N_{p,deck,uncr}$  must be substituted for  $N_{p,uncr}$  and the value 3,000 psi or 20.7 MPa must be substituted for 2,500 psi or 17.2 MPa.

**4.1.5 Requirements for Static Steel in Shear,  $V_{sa}$ :** The values of  $V_{sa}$  for a single anchor given in Tables 4 or 6 of this report must be used in lieu of the values of  $V_{sa}$  derived by calculation according to ACI 318 D.6.1.2.

The shear strength,  $V_{sa,deck}$ , of anchors installed in the soffit of sand lightweight or normal-weight concrete on steel deck floor and roof assemblies, as shown in Figure 7 of this report, is given in Table 9 of this report.

**4.1.6 Requirements for Static Concrete Breakout Strength in Shear,  $V_{cb}$  or  $V_{cbg}$ :** The nominal static concrete breakout strength in shear of a single anchor or a group of anchors,  $V_{cb}$  or  $V_{cbg}$ , must be calculated in accordance with ACI 318 D.6.2. The basic concrete breakout strength in shear of a single anchor in cracked concrete,  $V_b$ , must be calculated in accordance with ACI 318 D.6.2.2 using the value of  $d_a$ , given in Table 2 of this report, and the value  $l_e$ , given in Tables 4 or 6, must be taken no greater than  $h_{ef}$ . In no cases must  $l_e$  exceed  $8d_a$ .

For anchors installed in the soffit of sand lightweight or normal-weight concrete on steel deck floor and roof assemblies, as shown in Figure 7, calculation of the concrete breakout strength in accordance with ACI 318 D.6.2 is not required.

**4.1.7 Requirements for Static Concrete Pryout Strength of Anchor in Shear,  $V_{cp}$  or  $V_{cpq}$ :** The nominal static concrete pryout strength in shear of a single anchor or groups of anchors,  $V_{cp}$  or  $V_{cpq}$ , must be calculated in accordance with ACI 318 D.6.3, modified by using the value of  $k_{cp}$  provided in Tables 4 and 6 of this report and the value of  $N_{cb}$  or  $N_{cbq}$  as calculated in Section 4.1.3 of this report.

For anchors installed in the soffit of sand-lightweight or normal-weight concrete on steel deck floor and roof assemblies, as shown in Figure 7 of this report, calculation of the concrete pryout strength in accordance with ACI 318 D.6.3 is not required.

**4.1.8 Requirements for Minimum Member Thickness, Minimum Anchor Spacing and Minimum Edge Distance:** Values of  $s_{min}$  and  $c_{min}$  as given in Table 2 of this report must be used in lieu of ACI 318 D.8.1 and D.8.3. Minimum member thicknesses,  $h_{min}$ , as given in Tables 2 through 6 of this report, must be used in lieu of ACI 318 D.8.5.

For anchors installed in the soffit of sand-lightweight or normal-weight concrete on steel deck floor and roof

assemblies, the anchors must be installed in accordance with Figure 7 of this report and the minimum anchor spacing along the flute must be the greater of  $3h_{ef}$  or 1.5 times the flute width.

**4.1.9 Requirements for Critical Edge Distance and Splitting:** In applications where  $c < c_{ac}$  and supplemental reinforcement to control splitting of the concrete is not present, the concrete breakout strength in tension for uncracked concrete, calculated according to ACI 318 D.5.2, must be further multiplied by the factor  $\Psi_{cp,N}$  given by Eq-3:

$$\Psi_{cp,N} = c / c_{ac} \quad (\text{Eq-3})$$

whereby the factor  $\Psi_{cp,N}$  need not be taken as less than  $1.5h_{ef} / c_{ac}$ . For all other cases  $\Psi_{cp,N} = 1.0$ . Values for the critical edge distance  $c_{ac}$  must be taken from Tables 3 or 5 of this report.

**4.1.10 Requirements for Seismic Design:**

**4.1.10.1 General:** For load combinations including earthquake, the design must be performed according to ACI 318 D.3.3. For the 2012 IBC, Section 1905.1.9 is omitted. Modifications to ACI 318 D.3.3 must be applied under Section 1908.1.9 of the 2009 IBC or Section 1908.1.16 of the 2006 IBC, as applicable.

The nominal steel strength and the nominal concrete breakout strength for anchors in tension, and the nominal concrete breakout strength and pryout strength for anchors in shear, must be calculated according to ACI 318 D.5 and D.6, respectively, taking into account the corresponding values given in Tables 3 and 4 or Tables 5 and 6 of this report.

The carbon steel  $1/2$ -inch-  $5/8$ -inch- and  $3/4$ -inch-diameter (12.7, 15.9 and 19.1 mm) anchors, along with the  $3/8$ -inch-diameter (9.5 mm) anchor loaded in tension, comply with ACI 318 D.1 as ductile steel elements and must be designed in accordance with ACI 318-11 D.3.3.4, D.3.3.5, D.3.3.6, or D.3.3.7; ACI 318-08 D.3.3.4, D.3.3.5, or D.3.3.6; or ACI 318-05 D.3.3.4 or D.3.3.5, as applicable. The carbon steel  $3/8$ -inch-diameter (9.5 mm) anchor loaded in shear must be designed in accordance with ACI 318-11 D.3.3.5.3, ACI 318-08 D.3.3.5 or D.3.3.6 as brittle elements, or ACI 318-05 D.3.3.6 as brittle steel elements.

The stainless steel  $1/2$ -inch (12.7 mm) and  $5/8$ -inch (15.9 mm) anchors comply with ACI 318 D.1 as ductile steel elements and must be designed in accordance with ACI 318-11 D.3.3.4, D.3.3.5, D.3.3.6, or D.3.3.7; ACI 318-08 D.3.3.4, D.3.3.5, or D.3.3.6; or ACI 318-05 D.3.3.4 or D.3.3.5.

**4.1.10.2 Seismic Tension:** The nominal steel strength and nominal concrete breakout strength for anchors in tension must be calculated according to ACI 318 D.5.1 and D.5.2, as described in Sections 4.1.2 and 4.1.3 of this report. In accordance with ACI 318 D.5.3.2, the value for nominal pullout strength tension for seismic loads,  $N_{eq}$  or  $N_{p,deck,cr}$ , given in Table 3, 5 or 9 of this report, must be used in lieu of  $N_p$ . The values of  $N_{eq}$  must be adjusted for the concrete strength in accordance with Eq-4:

$$N_{eq,f_c} = N_{eq} \sqrt{\frac{f_c}{2,500}} \quad (\text{lb, psi}) \quad (\text{Eq-4})$$

$$N_{eq,f_c} = N_{eq} \sqrt{\frac{f_c}{17.2}} \quad (\text{N, MPa})$$

The value of  $N_{p,deck,cr}$  must be calculated according to Eq-4, whereby the value 3,000 psi or 20.7 MPa must be substituted for 2,500 psi or 17.2 MPa.

If no values for  $N_{eq}$  are given in Tables 3 or 5, the static design strength values govern. Section 4.1.4 provides additional requirements.

**4.1.10.3 Seismic Shear:** The nominal concrete breakout strength and pryout strength for anchors in shear must be calculated according to ACI 318 D.6.2 and D.6.3, as described in Sections 4.1.6 and 4.1.7 of this report. In accordance with ACI 318 D.6.1.2, the value for nominal steel strength in shear for seismic loads,  $V_{eq}$ , or  $V_{sa,deck}$ , given in Tables 4, 6 or 9 of this report, must be used in lieu of  $V_{sa}$ .

**4.1.11 Interaction of Tensile and Shear Forces:** For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318 D.7.

**4.1.12 Requirements for Sand Lightweight Concrete:** For ACI 318-11 and ACI 318-08, when anchors are used in sand-lightweight concrete, the modification factor  $\lambda_\alpha$  or  $\lambda$ , respectively, for concrete breakout strength must be taken as 0.6 in lieu of ACI 318-11 D.3.6 (2012 IBC) or ACI 318-08 D.3.4 (2009 IBC). In addition, the pullout strength  $N_{p,uncr}$ , and  $N_{eq}$  must be multiplied by 0.60, as applicable.

For ACI 318-05, the values  $N_b$ ,  $N_{eq}$ ,  $N_{p,uncr}$ , and  $V_b$  determined in accordance with this report must be multiplied by 0.60, in lieu of ACI 318 D.3.4.

For anchors installed in the soffit of sand-lightweight concrete-filled steel deck floor and roof assemblies, this reduction is not required.

## 4.2 Allowable Stress Design (ASD):

**4.2.1 General:** For anchors designed using load combinations in accordance with IBC Section 1605.3, allowable loads must be established using Eq-5 and Eq-6:

$$T_{allowable,ASD} = \phi N_r / \alpha \quad (\text{Eq-5})$$

and

$$V_{allowable,ASD} = \phi V_r / \alpha \quad (\text{Eq-6})$$

where

$$T_{allowable,ASD} = \text{Allowable tension load (lbf or kN).}$$

$$V_{allowable,ASD} = \text{Allowable shear load (lbf or kN).}$$

$\phi N_n$  = Lowest design strength of an anchor or anchor group in tension as determined in accordance with ACI 318 Appendix D with amendments in Section 4.1 of this report, or Section 1908.1.9 of the 2009 IBC, or Section 1908.1.16 of the 2006 IBC, as applicable.

$\phi V_n$  = Lowest design strength of an anchor or anchor group in shear as determined in accordance with ACI 318 Appendix D with amendments in Section 4.1 of this report, or Section 1908.1.9 of the 2009 IBC, or Section 1908.1.16 of the 2006 IBC, as applicable.

$\alpha$  = Conversion factor calculated as a weighted average of the load factors for the controlling load combination. In addition,  $\alpha$  must include all applicable factors to account for nonductile failure modes and required over-strength.

An example of allowable stress design values for illustrative purposes is shown in Table 7 of this report.

**4.2.2 Interaction of Tensile and Shear Forces:** In lieu of ACI 318 D.7.1, D.7.2 and D.7.3, interaction must be calculated as follows:

For shear loads  $V \leq 0.2 V_{allowable, ASD}$ , the full allowable load in tension,  $T_{allowable, ASD}$ , may be taken.

For tension loads  $T \leq 0.2 T_{allowable, ASD}$ , the full allowable load in shear,  $V_{allowable, ASD}$ , may be taken.

For all other cases, Eq-7 applies:

$$T/T_{allowable, ASD} + V/V_{allowable, ASD} \leq 1.2 \quad (\text{Eq-7})$$

For the OVERHEAD Trubolt+ Wedge Anchor, the influence of bending on the tension capacity when loaded in shear must be considered.

## 4.3 Installation:

Installation parameters are provided in Tables 2 and 8 and Figures 4, 5, and 6 of this report. Anchor locations must comply with this report and the plans and specifications approved by the code official. The Trubolt+ Wedge Anchors must be installed according to ITW's published instructions and this report. Holes must be predrilled in concrete with a compressive strength from 2,500 to 8,500 psi (17.2 to 58.6 MPa) at time of installation, using carbide-tipped masonry drill bits manufactured within the range of the maximum and minimum drill tip dimensions of ANSI Standard B212.15-1994. The nominal drill bit diameter must be equal to that of the anchor size. The minimum drilled hole depth,  $h_o$ , must comply with Table 2 of this report. Embedment, spacing, edge distance, and minimum concrete thickness must comply with Table 2. The predrilled holes must be cleaned to remove loose particles, using pressurized air or a vacuum. For the RED HEAD Trubolt+ Wedge Anchor, the hex nut and washer must be assembled on the end of the anchor, leaving the nut flush with the end of the anchor. For the OVERHEAD Trubolt+ Wedge Anchor, the coupling nut and washer must be assembled on the end of the anchor to obtain at least  $1/2$  inch (12.7 mm) thread engagement on the anchor). The anchors must be hammered into the predrilled hole to the required embedment depth in concrete. Where a fixture is installed, the anchors must be hammered through the fixture into the predrilled hole to the required embedment depth into the concrete. The nut must be tightened against the washer until the specified torque values listed in Table 2 are achieved.

For installation in the soffit of sand-lightweight or normal-weight concrete on steel deck floor and roof assemblies, the hole diameter in the steel deck must not exceed the diameter of the hole in the concrete by more than  $1/8$  inch (3.2 mm). For member thickness, edge distance, spacing restrictions, and installations torque values for installation into the soffit of sand lightweight or normal-weight concrete on steel deck floor and roof assemblies, see Figure 7, Table 8, and Section 4.1.8 of this report.

## 4.4 Special Inspection:

Periodic special inspection is required, in accordance with Section 1705.1.1 and Table 1705.3 of the 2012 IBC; Section 1704.15 and Table 1704.4 of the 2009 IBC; or Section 1704.13 of the 2006 IBC. The special inspector must make periodic inspections during anchor installation to verify anchor type, anchor dimensions, concrete type, concrete compressive strength, drill bit type, hole dimensions, hole cleaning procedures, edge distance, anchor spacing, concrete member thickness, anchor embedment, tightening torque, and adherence to the manufacturer's published installation instructions. The special inspector must be present as often as required in

accordance with the statement of special inspection. Under the IBC, additional requirements as set forth in Sections 1705, 1706, and 1707 must be observed, where applicable.

## 5.0 CONDITIONS OF USE

The Trubolt+ Wedge Anchors described in this report comply with, or are suitable alternatives to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions.

- 5.1 The anchors must be installed in accordance with ITW's published instructions and this report. In case of conflicts, this report governs.
- 5.2 Anchor sizes, dimensions, and installation parameters are as set forth in this report.
- 5.3 The anchors are limited to installation in cracked and uncracked, normal-weight or sand-lightweight concrete having a specified compressive strength,  $f'_c$ , of 2,500 psi to 8,500 psi (17.2 MPa to 58.6 MPa). The anchors may also be installed in cracked and uncracked normal-weight or sand-lightweight concrete over profile steel deck having a minimum specified compressive strength,  $f'_c$ , of 3,000 psi (20.7 MPa).
- 5.4 The values of  $f'_c$  used for calculation purposes must not exceed 8,000 psi (55.0 MPa).
- 5.5 Strength design values must be established in accordance with Section 4.1 of this report.
- 5.6 Allowable design values must be established in accordance with Section 4.2 of this report.
- 5.7 Anchor spacing, edge distance, and minimum member thickness must comply with Tables 2 and 8 and Figures 4, 5, and 6 of this report.
- 5.8 Prior to installation, calculations and details justifying that the applied loads comply with this report must be submitted to the code official for approval. The calculations and details must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.
- 5.9 Since an ICC-ES acceptance criteria for evaluating data to determine the performance of expansion anchors subjected to fatigue or shock loading is unavailable at this time, the use of these anchors under such conditions is beyond the scope of this report.
- 5.10 Anchors may be installed in regions of concrete where cracking has occurred or where analysis indicates cracking may occur ( $f_t > f_r$ ), subject to the conditions of this report.

5.11 Anchors may be used to resist short-term loading due to wind or seismic forces, subject to the conditions of this report.

5.12 Where not otherwise prohibited in the code, Trubolt+ Wedge Anchors are permitted for use with fire-resistance-rated construction provided that at least one of the following conditions is fulfilled:

- Anchors are used to resist wind or seismic forces only.
- Anchors that support a fire-resistance-rated envelope or a fire-resistance-rated membrane are protected by approved fire-resistance-rated materials, or have been evaluated for resistance to fire exposure in accordance with recognized standards.
- Anchors are used to support nonstructural elements.

5.13 Use of the zinc plated, carbon steel anchors is limited to dry, interior locations.

5.14 Special inspections are provided in accordance with Section 4.4 of this report.

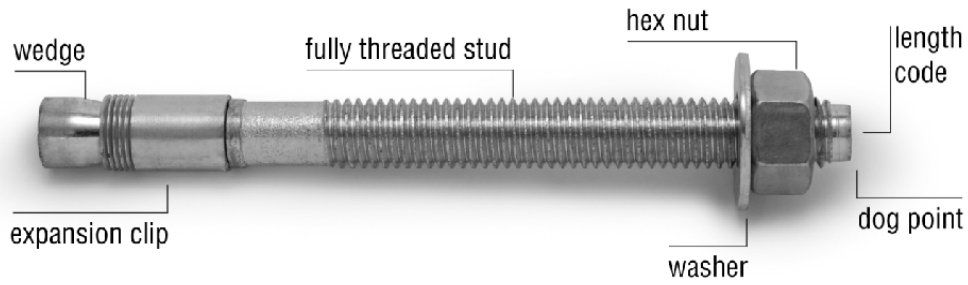
5.15 The anchors are manufactured in the USA; under a quality control program with inspections by ICC-ES.

## 6.0 EVIDENCE SUBMITTED

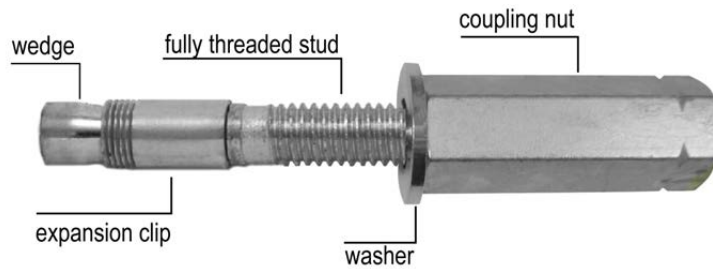
Data complying with the ICC-ES Acceptance Criteria for Mechanical Anchors in Concrete Elements (AC193), dated June 2012, editorially revised April 2014, for use in cracked and uncracked concrete, including optional tests for seismic tension and shear; profile steel deck soffit tests; and quality control documentation.

## 7.0 IDENTIFICATION

The anchors are identified by their dimensional characteristics and the anchor size, and by a length identification marking stamped on the anchor, as indicated in Table 1 of this report. The anchors have the length identification marking underlined on the anchor head, as illustrated in Figure 3 of this report, and this is visible after installation for verification. Packages are identified with the anchor name, material (carbon or stainless) type and size; the manufacturer's name (ITW Red Head, ITW Brands, or ITW Buildex) and address; and the evaluation report number (ESR-2427).



**FIGURE 1—ITW RED HEAD TRUBOLT+ WEDGE ANCHOR (Carbon and Stainless Steel)**



**FIGURE 2—OVERHEAD TRUBOLT+ WEDGE ANCHOR**

**TABLE 1—LENGTH IDENTIFICATION MARKINGS<sup>1</sup>**

LENGTH (inches)	ID MARKING ON ANCHOR HEAD												
	C	D	E	F	G	H	I	J	K	L	M	N	O
From	2½	3	3½	4	4½	5	5½	6	6½	7	7½	8	8½
Up to, but not including	3	3½	4	4½	5	5½	6	6½	7	7½	8	8½	9

For SI: 1 inch = 25.4 mm.

<sup>1</sup>Figure 3 shows a typical marking.



**FIGURE 3—TRUBOLT+ WEDGE ANCHOR LENGTH IDENTIFICATION MARKING**

**TABLE 2—ITW RED HEAD TRUBOLT+ WEDGE ANCHOR AND OVERHEAD TRUBOLT+ WEDGE ANCHOR INSTALLATION INFORMATION (CARBON STEEL AND STAINLESS STEEL)<sup>1</sup>**

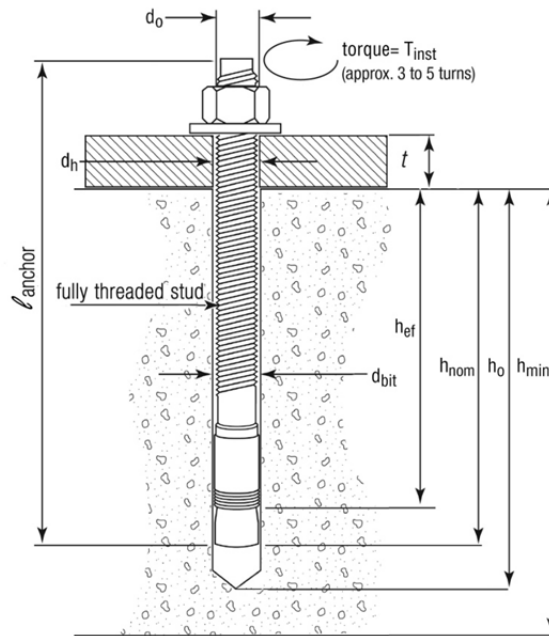
PARAMETER	NOTATION	UNITS	NOMINAL ANCHOR DIAMETER (inch)									
			$\frac{3}{8}$		$\frac{1}{2}$		$\frac{5}{8}$		$\frac{3}{4}$			
Anchor outer diameter	$d_a[d_o]^3$	inches	0.361		0.5		0.615		0.7482			
Nominal carbide bit diameter	$d_{bit}$	inches	$\frac{3}{8}$		$\frac{1}{2}$		$\frac{5}{8}$		$\frac{3}{4}$			
Effective embedment depth	$h_{ef}$	inches	$1\frac{5}{8}$		2		$3\frac{1}{4}$		$2\frac{3}{4}$   $4\frac{1}{4}$		$3\frac{3}{4}$	
Minimum anchor embedment depth	$h_{nom}$	inches	2		$2\frac{1}{2}$		$3\frac{3}{4}$		$3\frac{1}{4}$   $4\frac{3}{4}$		$4\frac{3}{8}$	
Minimum hole depth <sup>1</sup>	$h_o$	inches	$2\frac{1}{4}$		$2\frac{3}{4}$		4		$3\frac{1}{2}$   5		$4\frac{5}{8}$	
Minimum concrete member thickness <sup>1</sup>	$h_{min}$	inches	4	5	4	6	6	8	6	$6\frac{1}{4}$	7	8
Critical edge distance <sup>1</sup>	$c_{ac}$	In.	5	3	6	6	$7\frac{1}{2}$	6	$7\frac{1}{2}$	$6\frac{1}{2}$	12	10
Minimum anchor spacing <sup>1</sup>	$s_{min}$	In.	$3\frac{1}{2}$	$2\frac{1}{2}$	6	$5\frac{3}{4}$	4	$5\frac{3}{4}$	8	6	6	6
Minimum edge distance <sup>2</sup>	$c_{min}$	In.	3		6			$7\frac{1}{2}$		5	$7\frac{1}{2}$	$7\frac{1}{2}$
Minimum overall anchor length	$l_{anchor}$	inches	$2\frac{1}{2}$		$3\frac{3}{4}$		$4\frac{1}{2}$		$4\frac{1}{4}$   6		$5\frac{1}{2}$	
Installation torque	$T_{inst}$	ft-lb	30		45		90		110			
Minimum diameter of hole in fastened part	$d_h$	inches	$\frac{1}{2}$		$\frac{5}{8}$		$\frac{3}{4}$		$\frac{7}{8}$			

For SI: 1 inch = 25.4 mm, 1 ft-lb = 1.356 N-m.

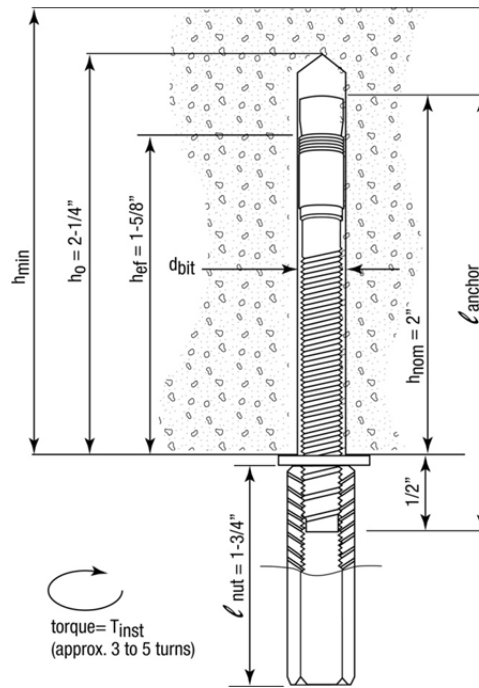
<sup>1</sup>Stainless steel anchors are available in  $\frac{1}{2}$ -inch and  $\frac{5}{8}$ -inch-diameters. The OVERHEAD version is available in a carbon steel  $\frac{3}{8}$ -inch-diameter.

<sup>2</sup>For installation of the carbon steel anchors in the soffit of concrete on steel deck floor or roof assemblies, see Figure 7. Anchors in the lower and in the upper flute may be installed with a maximum 1-inch offset in either direction from the center of the flute. In addition, anchors must have an axial spacing along the flute equal to the greater of  $3h_{ef}$  or 1.5 times the flute width.

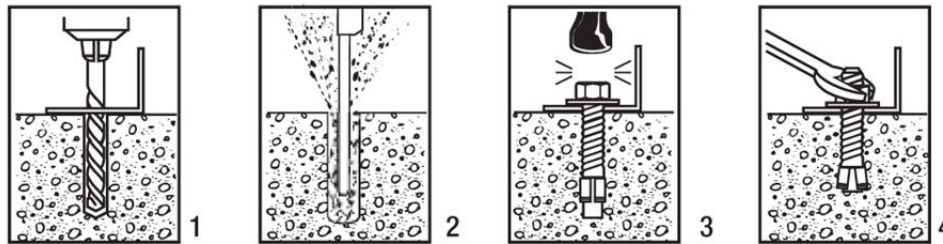
<sup>3</sup>The notation in brackets is for the 2006 IBC.



**FIGURE 4—ITW RED HEAD TRUBOLT+ WEDGE ANCHOR (INSTALLED)**



**FIGURE 5—OVERHEAD TRUBOLT+ WEDGE ANCHOR (INSTALLED),  
3/8 INCH NOMINAL ANCHOR DIAMETER (d<sub>a</sub>)**



1. Select a carbide drill bit with a diameter equal to the anchor diameter. Drill hole 1/4" deeper than anchor embedment.
2. Clean hole with pressurized air or vacuum to remove any excess dust/debris.
3. Using the washer and nut provided, assemble the anchor, leaving nut one half turn from the end of anchor to protect threads. Drive anchor through fixture to be fastened until washer is flush to surface of fixture.
4. Expand anchor by tightening nut to the specified setting torque (approx 3-5 turns).

**FIGURE 6—INSTALLATION INSTRUCTIONS**



**TABLE 3—ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHOR AND OVERHEAD TRUBOLT+ WEDGE ANCHOR TENSION DESIGN INFORMATION<sup>1,2,3,9</sup>**

CHARACTERISTIC	SYMBOL	UNITS	NOMINAL ANCHOR DIAMETER (inch) <sup>6</sup>									
			<sup>3</sup> / <sub>8</sub>		<sup>1</sup> / <sub>2</sub>		<sup>5</sup> / <sub>8</sub>		<sup>3</sup> / <sub>4</sub>			
Anchor category	1, 2 or 3	—	1		1		1		1			
Minimum effective embedment depth	$h_{ef}$	In.	1 <sup>5</sup> / <sub>8</sub>		2		3 <sup>1</sup> / <sub>4</sub>		2 <sup>3</sup> / <sub>4</sub> , 4 <sup>1</sup> / <sub>4</sub>		3 <sup>3</sup> / <sub>4</sub>	
Minimum concrete member thickness	$h_{min}$	In.	4	5	4	6	6	8	6	6 <sup>1</sup> / <sub>4</sub>	7	8
Critical edge distance	$c_{ac}$	In.	5	3	6	6	7 <sup>1</sup> / <sub>2</sub>	6	7 <sup>1</sup> / <sub>2</sub>	6 <sup>1</sup> / <sub>2</sub>	12	10
<b>Data for Steel Strengths – Tension</b>												
Minimum specified yield strength	$f_y$	psi	60,000		55,000		55,000		55,000		55,000	
Minimum specified ultimate strength	$f_{ult}$	psi	75,000		75,000		75,000		75,000		75,000	
Effective tensile stress area (neck)	$A_{se,N} [A_{se}]^{\beta}$	in <sup>2</sup>	0.056		0.119		0.183		0.266		0.266	
Steel strength in tension	$N_{sa}$	lbf	4,200		8,925		13,725		19,950		19,950	
Strength reduction factor $\phi$ for tension, steel failure modes <sup>4</sup>	$\phi$	—	0.75		0.75		0.75		0.75		0.75	
<b>Data for Concrete Breakout Strengths in Tension</b>												
Effectiveness factor - uncracked concrete	$k_{uncr}$	—	24		24		24		24		24	
Effectiveness factor - cracked concrete	$k_{cr}$	—	17		17		17		17		17	
Modification factor for cracked and uncracked concrete <sup>5</sup>	$\psi_{c,N}$	—	1.0		1.0		1.0		1.0		1.0	
Strength reduction factor $\phi$ for tension, concrete failure modes, Condition B <sup>4</sup>	$\phi$	—	0.65		0.65		0.65		0.65		0.65	
<b>Data for Pullout Strengths</b>												
Pullout strength, uncracked concrete	$N_{p,uncr}$	lbf	See Footnote 7		See Footnote 7		6,540		5,430		8,900	
Pullout strength, cracked concrete	$N_{p,cr}$	lbf	See Footnote 7		See Footnote 7		See Footnote 7		See Footnote 7		See Footnote 7	
Pullout strength for seismic loads	$N_{eq}$	lbf	See Footnote 7		See Footnote 7		See Footnote 7		See Footnote 7		6,715	
Strength reduction factor $\phi$ for tension, pullout failure modes, Condition B <sup>4</sup>	$\phi$	—	See Footnote 7		0.65		0.65		0.65		See Footnote 7	
<b>Additional Anchor Data</b>												
Axial stiffness in service load range in uncracked concrete	$\beta_{uncr}$	lbf /in	100,000		250,000		250,000		250,000		250,000	
Axial stiffness in service load range in cracked concrete	$\beta_{cr}$	lbf /in	40,000		20,000		20,000		20,000		20,000	

For SI: 1 inch = 25.4 mm, 1 in<sup>2</sup> = 645.16 mm<sup>2</sup>, 1 lbf = 4.45 N, 1 psi = 0.006895 MPa, 1 lbf · 10<sup>2</sup>/in = 17,500 N/m.

<sup>1</sup>The data in this table is intended to be used with the design provisions of ACI 318 Appendix D; for anchors resisting seismic load combinations, the additional requirements of ACI 318 D.3.3 shall apply.

<sup>2</sup>Installation must comply with the manufacturers printed installation instructions and details.

<sup>3</sup>The <sup>3</sup>/<sub>8</sub>-, <sup>1</sup>/<sub>2</sub>-, <sup>5</sup>/<sub>8</sub>-, and <sup>3</sup>/<sub>4</sub>-inch diameter Trubolt + Wedge Anchors are ductile steel elements as defined by ACI 318 D.1.

<sup>4</sup>All values of  $\phi$  apply to the load combinations of IBC Section 1605.2 or ACI 318 Section 9.2. If the load combinations of Appendix C are used, then the appropriate value of  $\phi$  must be determined in accordance with ACI 318-11 D.4.4 (ACI 318-08 and -05 D.4.5). For installations where reinforcement that complies with ACI 318 Appendix D requirements for Condition A is present, the appropriate  $\phi$  factor must be determined in accordance with ACI 318-11 D.4.3 (ACI 318-08 and -05 D.4.4).

<sup>5</sup>For all design cases  $\psi_{c,N} = 1.0$ . The appropriate effectiveness factor for cracked concrete ( $k_{cr}$ ) or uncracked concrete ( $k_{uncr}$ ) must be used.

<sup>6</sup>The actual diameter for the <sup>3</sup>/<sub>8</sub>-inch diameter anchor is 0.361 inch, for the <sup>5</sup>/<sub>8</sub>-inch diameter anchor is 0.615-inch, and for the <sup>3</sup>/<sub>4</sub>-inch diameter anchor is 0.7482-inch.

<sup>7</sup>Anchor pullout strength does not control anchor design. Determine steel and concrete capacities only.

<sup>8</sup>The notation in brackets is for the 2006 IBC.

<sup>9</sup>The OVERHEAD version is available in a carbon steel <sup>3</sup>/<sub>8</sub>-inch-diameter only.

**TABLE 4—ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHOR AND OVERHEAD TRUBOLT+ WEDGE ANCHOR SHEAR DESIGN INFORMATION<sup>1,2,3,8</sup>**

CHARACTERISTIC	SYMBOL	UNITS	NOMINAL ANCHOR DIAMETER (inch) <sup>5</sup>									
			<sup>3</sup> / <sub>8</sub>		<sup>1</sup> / <sub>2</sub>		<sup>5</sup> / <sub>8</sub>		<sup>3</sup> / <sub>4</sub>			
Anchor category	1, 2 or 3	—	1		1		1		1			
Minimum effective embedment depth	$h_{ef}$	In.	<sup>1</sup> / <sub>8</sub>		2		3 <sup>1</sup> / <sub>4</sub>		2 <sup>3</sup> / <sub>4</sub> , 4 <sup>1</sup> / <sub>4</sub>		3 <sup>3</sup> / <sub>4</sub>	
Minimum concrete member thickness	$h_{min}$	In.	4	5	4	6	6	8	6	6 <sup>1</sup> / <sub>4</sub>	7	8
Critical edge distance	$c_{ac}$	In.	5	3	6	6	7 <sup>1</sup> / <sub>2</sub>	6	7 <sup>1</sup> / <sub>2</sub>	6 <sup>1</sup> / <sub>2</sub>	12	10
<b>Data for Steel Strengths – Shear</b>												
Minimum specified yield strength	$f_y$	psi	60,000		55,000		55,000		55,000		55,000	
Minimum specified ultimate strength	$f_{uta}$	psi	75,000		75,000		75,000		75,000		75,000	
Effective shear stress area (thread)	$A_{se,V} [A_{se}]^7$	in <sup>2</sup>	0.075		0.142		0.217		0.332			
Steel strength in shear, uncracked or cracked concrete <sup>6</sup>	$V_{sa}$	lbf	1,830		5,175		8,955		14,970			
Steel strength in shear - seismic loads	$V_{eq}$	lbf	1,545		5,175		8,955		11,775			
Strength reduction factor $\phi$ for shear, steel failure modes <sup>4</sup>	$\phi$	—	0.60		0.65		0.65		0.65		0.65	
<b>Data for Concrete Breakout and Concrete Pryout Strengths – Shear</b>												
Coefficient for pryout strength	$k_{cp}$	—	1.0		1.0		2.0		2.0		2.0	
Load-bearing length of anchor	$l_e$	in	<sup>1</sup> / <sub>8</sub>		2		3 <sup>1</sup> / <sub>4</sub>		2 <sup>3</sup> / <sub>4</sub> , 4 <sup>1</sup> / <sub>4</sub>		3 <sup>3</sup> / <sub>4</sub>	
Strength reduction factor $\phi$ for shear, concrete failure modes, Condition B <sup>4</sup>	$\phi$	—	0.70		0.70		0.70		0.70		0.70	

For **SI**: 1 inch = 25.4 mm, 1 in<sup>2</sup> = 645.16 mm<sup>2</sup>, 1 lbf = 4.45 N, 1 psi = 0.006895 MPa, 1 lbf · 10<sup>2</sup>/in = 17,500 N/m.

<sup>1</sup>The data in this table is intended to be used with the design provisions of ACI 318 Appendix D; for anchors resisting seismic load combinations, the additional requirements of ACI 318 D.3.3 shall apply.

<sup>2</sup>Installation must comply with the manufacturers printed installation instructions and details.

<sup>3</sup>The <sup>1</sup>/<sub>2</sub>-, <sup>5</sup>/<sub>8</sub>-, and <sup>3</sup>/<sub>4</sub>-inch diameter Trubolt + Wedge Anchors are ductile steel elements as defined by ACI 318 D.1. The <sup>3</sup>/<sub>8</sub>" diameter Trubolt + is considered brittle under shear loading.

<sup>4</sup>All values of  $\phi$  apply to the load combinations of IBC Section 1605.2, ACI 318 Section 9.2. If the load combinations of ACI 318 Appendix C are used, the appropriate value of  $\phi$  must be determined in accordance with ACI 318-11 D.4.4 (ACI 318-08 and -05 D.4.5). For installations where reinforcement that complies with ACI 318 Appendix D requirements for Condition A is present, the appropriate  $\phi$  factor must be determined in accordance with ACI 318-11 D.4.3 (ACI 318-08 and -05 D.4.4).

<sup>5</sup>The actual diameter for the <sup>3</sup>/<sub>8</sub>" diameter anchor is 0.361-inch, for the <sup>5</sup>/<sub>8</sub>" diameter anchor is 0.615-inch, and for the <sup>3</sup>/<sub>4</sub>" diameter anchor is 0.7482".

<sup>6</sup>Steel strength in shear values are based on test results per ACI 355.2, Section 9.4 and must be used for design.

<sup>7</sup>The notation in brackets is for the 2006 IBC.

<sup>8</sup>The OVERHEAD version is available in a carbon steel <sup>3</sup>/<sub>8</sub>-inch-diameter only.

**TABLE 5—ITW RED HEAD STAINLESS STEEL TRUBOLT+ WEDGE ANCHOR TENSION DESIGN INFORMATION<sup>1,2,3</sup>**

CHARACTERISTIC	SYMBOL	UNITS	NOMINAL ANCHOR DIAMETER (inch) <sup>6</sup>							
			<sup>1</sup> / <sub>2</sub>			<sup>5</sup> / <sub>8</sub>				
Anchor category	1, 2 or 3	—	1			1				
Minimum effective embedment depth	$h_{ef}$	In.	2		3 <sup>1</sup> / <sub>4</sub>		2 <sup>3</sup> / <sub>4</sub>		4 <sup>1</sup> / <sub>4</sub>	
Minimum concrete member thickness	$h_{min}$	In.	4	6	6	8	6	6 <sup>1</sup> / <sub>4</sub>		
Critical edge distance	$c_{ac}$	In.	6	6	7 <sup>1</sup> / <sub>2</sub>	6	7 <sup>1</sup> / <sub>2</sub>	6 <sup>1</sup> / <sub>2</sub>		
Data for Steel Strengths – Tension										
Minimum specified yield strength	$f_y$	psi	65,000				65,000			
Minimum specified ultimate strength	$f_{uta}$	psi	100,000				100,000			
Effective tensile stress area (neck)	$A_{se,N} [A_{se}]^8$	in <sup>2</sup>	0.119				0.183			
Steel strength in tension	$N_{sa}$	lbf	11,900				18,300			
Strength reduction factor $\phi$ for tension, steel failure modes <sup>4</sup>	$\phi$	—	0.75				0.75			
Data for Concrete Breakout Strengths in Tension										
Effectiveness factor - uncracked concrete	$k_{unscr}$	—	24				24			
Effectiveness factor - cracked concrete	$k_{cr}$	—	17				17			
Modification factor for cracked and uncracked concrete <sup>5</sup>	$\psi_{c,N}$	—	1.0				1.0			
Strength reduction factor $\phi$ for tension, concrete failure modes, Condition B <sup>4</sup>	$\phi$	—	0.65				0.65			
Data for Pullout Strengths										
Pullout strength, uncracked concrete	$N_{p,unscr}$	lbf	See Footnote 7		6,540	5,430	8,900			
Pullout strength, cracked concrete	$N_{p,cr}$	lbf	See Footnote 7				See Footnote 7			
Pullout strength for seismic loads	$N_{eq}$	lbf	2,345		See Footnote 7		See Footnote 7			
Strength reduction factor $\phi$ for tension, pullout failure modes, Condition B <sup>4</sup>	$\phi$	—	0.65				0.65			
Additional Anchor Data										
Axial stiffness in service load range in uncracked concrete	$\beta_{unscr}$	lbf /in	250,000				250,000			
Axial stiffness in service load range in cracked concrete	$\beta_{cr}$	lbf /in	20,000				20,000			

For SI: 1 inch = 25.4 mm, 1 in<sup>2</sup> = 645.16 mm<sup>2</sup>, 1 lbf = 4.45 N, 1 psi = 0.006895 MPa, 1 lbf · 10<sup>2</sup>/in = 17,500 N/m.

<sup>1</sup>The data in this table is intended to be used with the design provisions of ACI 318 Appendix D; for anchors resisting seismic load combinations, the additional requirements of ACI 318 D.3.3 shall apply.

<sup>2</sup>Installation must comply with the manufacturers printed installation instructions and details.

<sup>3</sup>The <sup>1</sup>/<sub>2</sub>- and <sup>5</sup>/<sub>8</sub>-inch diameter Trubolt + Wedge Anchors are ductile steel elements as defined by ACI 318 D.1.

<sup>4</sup>All values of  $\phi$  apply to the load combinations of IBC Section 1605.2 or ACI 318 Section 9.2. If the load combinations of Appendix C are used, the appropriate value of  $\phi$  must be determined in accordance with ACI 318-11 D.4.4 (ACI 318-08 and -05 D.4.5). For installations where reinforcement that complies with ACI 318 Appendix D requirements for Condition A is present, the appropriate  $\phi$  factor must be determined in accordance with ACI 318-11 D.4.3 (ACI 318-08 and -05 D.4.4).

<sup>5</sup>For all design cases  $\psi_{c,N} = 1.0$ . The appropriate effectiveness factor for cracked concrete ( $k_{cr}$ ) or uncracked concrete ( $k_{unscr}$ ) must be used.

<sup>6</sup>The actual diameter for the <sup>5</sup>/<sub>8</sub>-inch diameter anchor is 0.615-inch.

<sup>7</sup>Anchor pullout strength does not control anchor design. Determine steel and concrete capacities only.

<sup>8</sup>The notation in brackets is for the 2006 IBC.

**TABLE 6—ITW RED HEAD STAINLESS STEEL TRUBOLT+ WEDGE ANCHOR  
SHEAR DESIGN INFORMATION<sup>1,2,3</sup>**

CHARACTERISTIC	SYMBOL	UNITS	NOMINAL ANCHOR DIAMETER (inch) <sup>5</sup>						
			<sup>1</sup> / <sub>2</sub>			<sup>5</sup> / <sub>8</sub>			
Anchor category	1, 2 or 3	—	1			1			
Minimum effective embedment depth	$h_{ef}$	In.	2		3 <sup>1</sup> / <sub>4</sub>		2 <sup>3</sup> / <sub>4</sub>		4 <sup>1</sup> / <sub>4</sub>
Minimum concrete member thickness	$h_{min}$	In.	4	6	6	8	6	6 <sup>1</sup> / <sub>4</sub>	
Critical edge distance	$c_{ac}$	In.	6	6	7 <sup>1</sup> / <sub>2</sub>	6	7 <sup>1</sup> / <sub>2</sub>	6 <sup>1</sup> / <sub>2</sub>	
Data for Steel Strengths - Shear									
Minimum specified yield strength	$f_y$	psi	65,000			65,000			
Minimum specified ultimate strength	$f_{uta}$	psi	100,000			100,000			
Effective shear stress area (thread)	$A_{se,v} [A_{se}]^7$	in <sup>2</sup>	0.142			0.217			
Steel strength in shear, uncracked or cracked concrete <sup>6</sup>	$V_{sa}$	lbf	7,265			10,215			
Steel strength in shear - seismic loads	$V_{eq}$	lbf	5,805			8,105			
Strength reduction factor $\phi$ for shear, steel failure modes <sup>4</sup>	$\phi$	—	0.65			0.65			
Data for Concrete Breakout and Concrete Pryout Strengths - Shear									
Coefficient for pryout strength	$k_{cp}$	—	1.0		2.0		2.0		
Load-bearing length of anchor	$l_e$	in	2		3 <sup>1</sup> / <sub>4</sub>		2 <sup>3</sup> / <sub>4</sub>		4 <sup>1</sup> / <sub>4</sub>
Strength reduction factor $\phi$ for shear, concrete failure modes, Condition B <sup>4</sup>	$\phi$	—	0.70			0.70			

For **SI**: 1 inch = 25.4 mm, 1 in<sup>2</sup> = 645.16 mm<sup>2</sup>, 1 lbf = 4.45 N, 1 psi = 0.006895 MPa, 1 lbf · 10<sup>2</sup>/in = 17,500 N/m.

<sup>1</sup>The data in this table is intended to be used with the design provisions of ACI 318 Appendix D; for anchors resisting seismic load combinations, the additional requirements of ACI 318 D.3.3 shall apply.

<sup>2</sup>Installation must comply with the manufacturers printed installation instructions and details.

<sup>3</sup>The <sup>1</sup>/<sub>2</sub>- and <sup>5</sup>/<sub>8</sub>-inch diameter Trubolt + Wedge Anchors are ductile steel elements as defined by ACI 318 D.1.

<sup>4</sup>All values of  $\phi$  apply to the load combinations of IBC Section 1605.2 or ACI 318 Section 9.2. If the load combinations of Appendix C are used, the appropriate value of  $\phi$  must be determined in accordance with ACI 318-11 D.4.4 (ACI 318-08 and -05 D.4.5). For installations where reinforcement that complies with ACI 318 Appendix D requirements for Condition A is present, the appropriate  $\phi$  factor must be determined in accordance with ACI 318-11 D.4.3 (ACI 318-08 and -05 D.4.4).

<sup>5</sup>The actual diameter for the <sup>5</sup>/<sub>8</sub>" diameter anchor is 0.615-inch.

<sup>6</sup>Steel strength in shear values are based on test results per ACI 355.2, Section 9.4 and must be used for design.

<sup>7</sup>The notation in brackets is for the 2006 IBC.

**TABLE 7—EXAMPLE ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHOR AND OVERHEAD TRUBOLT+ WEDGE ANCHOR ALLOWABLE STRESS DESIGN (ASD) VALUES FOR ILLUSTRATIVE PURPOSES<sup>1,2,3,4,5,6,7,8,9,10</sup>**

ANCHOR NOTATION	ANCHOR EMBEDMENT DEPTH (inches), $h_{nom}$	EFFECTIVE EMBEDMENT DEPTH (inches), $h_{ef}$	ALLOWABLE TENSION LOAD (lbs)
$3/8$	2	$1^{5/8}$	1,090
$1/2$	$2^{1/2}$	2	1,490
	$3^{3/4}$	$3^{1/4}$	2,870
$5/8$	$3^{1/4}$	$2^{3/4}$	2,385
	$4^{3/4}$	$4^{1/4}$	3,910
$3/4$	$4^{3/8}$	$3^{3/4}$	3,825

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

**Design assumptions:**

<sup>1</sup>Single anchor with static tension load only.

<sup>2</sup>Concrete determined to remain uncracked for the life of the anchorage.

<sup>3</sup>Load combinations are in accordance with ACI 318 Section 9.2 and no seismic loading.

<sup>4</sup>Thirty percent dead load and 70 percent live load, controlling load combination  $1.2D + 1.6L$ .

<sup>5</sup>Calculation of weighted average for  $\alpha$ :  $1.2D + 1.6L = 1.2(0.3) + 1.6(0.7) = 1.48$ .

<sup>6</sup> $f'_c = 2,500$  psi (normal-weight concrete).

<sup>7</sup> $C_{a1} = C_{a2} > C_{ac}$ .

<sup>8</sup> $h \geq h_{min}$ .

<sup>9</sup>Values are for Condition B where supplementary reinforcement in accordance with ACI 318-11 D.4.3 is not provided.

<sup>10</sup>The Overhead Trubolt+ Wedge version is available in a carbon steel  $3/8$ -inch-diameter only.

**Illustrative Procedure to Calculate Allowable Stress Design Tension Value:**

RED HEAD Carbon Steel Trubolt+ Wedge Anchor  $1/2$  inch diameter using an effective embedment of  $3^{1/4}$  inches, assuming the given conditions in Table 3, in accordance with ACI 318-11 Appendix D and this report.

PROCEDURE	CALCULATION
Step 1 Calculate steel strength of a single anchor in tension per ACI 318 D 5.1.2, Table 3 of this report	$\phi N_{sa} = \phi N_{sa}$ $= 0.75 * 8,925$ <b>= 6,694 lbs steel strength</b>
Step 2 Calculate concrete breakout strength of a single anchor in tension per ACI 318 D 5.2.1, Table 3 of this report	$N_b = k_{unscr} * \lambda_a * \sqrt{f'_c} * h_{ef}^{1.5}$ $= 24 * 1.0 * \sqrt{2,500} * 3.25^{1.5}$ $= 7,031$ lbs  $\phi N_{cb} = \phi A_{NC}/A_{NC0} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b$ $= 0.65 * (95/95) / 1.0 * 1.0 * 1.0 * 7,031$ $= 0.65 * 7,031$ <b>= 4,570 lbs concrete breakout strength</b>
Step 3 Calculate pullout strength in tension per ACI 318 D 5.3.2 and Table 3 of this report	$\phi N_{pn} = \phi N_{p,unscr} \psi_{c,P} (f'_{c,act}/2,500)^{\eta}$ $= 0.65 * 6,540 * 1.0 * 1.0^{0.5}$ <b>= 4,251 lbs pullout strength</b>
Step 4 Determine controlling resistance strength in tension per ACI 318 D 4.1.1	<b>= 4,251 lbs controlling resistance</b>
Step 5 Calculate allowable stress design conversion factor for loading condition per ACI 318 Section 9.2:	$\alpha = 1.2D + 1.6L$ $= 1.2(0.3) + 1.6(0.7)$ <b>= 1.48</b>
Step 6 Calculate allowable stress design value per Section 4.2 of this report	$T_{allowable,ASD} = \phi N_n / \alpha$ $= 4,251 / 1.48$ <b>= 2,870 lbs allowable stress design</b>

**TABLE 8—ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHOR AND OVERHEAD TRUBOLT+ WEDGE ANCHOR INSTALLATION INFORMATION FOR ANCHORS LOCATED IN THE SOFFIT OF CONCRETE FILL ON METAL DECK FLOOR AND ROOF ASSEMBLIES<sup>1,3</sup>**

PARAMETER	SYMBOL	UNITS	NOMINAL ANCHOR DIAMETER (inch)				
			<sup>3</sup> / <sub>8</sub>	<sup>1</sup> / <sub>2</sub>	<sup>5</sup> / <sub>8</sub>		
Anchor outer diameter	$d_a[d_o]^2$	inches	0.361	0.5	0.615		
Nominal carbide bit diameter	$d_{bit}$	inches	<sup>3</sup> / <sub>8</sub>	<sup>1</sup> / <sub>2</sub>	<sup>5</sup> / <sub>8</sub>		
Location of Installation	-	-	upper and lower flute	upper and lower flute	lower flute	lower flute	
Minimum effective embedment depth	$h_{ef}$	Inches	<sup>1</sup> / <sub>2</sub>	2	<sup>3</sup> / <sub>4</sub>	<sup>2</sup> / <sub>4</sub>	<sup>4</sup> / <sub>4</sub>
Anchor embedment depth	$h_{nom}$	Inches	2	<sup>2</sup> / <sub>2</sub>	<sup>3</sup> / <sub>4</sub>	<sup>3</sup> / <sub>4</sub>	<sup>4</sup> / <sub>4</sub>
Minimum hole depth	$h_o$	Inches	<sup>2</sup> / <sub>4</sub>	<sup>2</sup> / <sub>4</sub>	4	<sup>3</sup> / <sub>2</sub>	5
Minimum overall anchor length	$l_{anchor}$	Inches	<sup>2</sup> / <sub>2</sub>	<sup>3</sup> / <sub>4</sub>	<sup>4</sup> / <sub>2</sub>	<sup>4</sup> / <sub>4</sub>	6
Installation torque	$T_{inst}$	ft-lb	30	45		90	
Minimum diameter of hole in fastened part	$d_h$	inches	<sup>1</sup> / <sub>2</sub>	<sup>5</sup> / <sub>8</sub>		<sup>3</sup> / <sub>4</sub>	

For **SI**: 1 inch = 25.4 mm, 1 ft-lb = 1.356 N-m.

<sup>1</sup> Anchors in the lower and upper flute may be installed with a maximum 1-inch offset in either direction from the center of the flute. In addition, anchors must have an axial spacing along the flute equal to the greater of  $3h_{ef}$  or 1.5 times the flute width.

<sup>2</sup> The notation in brackets is for the 2006 IBC.

<sup>3</sup> The Overhead Trubolt+ Wedge version is available in a carbon steel <sup>3</sup>/<sub>8</sub>-inch-diameter only.

**TABLE 9—ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHOR AND OVERHEAD TRUBOLT+ WEDGE ANCHOR DESIGN INFORMATION FOR ANCHORS LOCATED IN THE SOFFIT OF CONCRETE FILL ON METAL DECK FLOOR AND ROOF ASSEMBLIES<sup>1,2</sup>**

CHARACTERISTIC	SYMBOL	UNITS	NOMINAL ANCHOR DIAMETER (inch)				
			<sup>3</sup> / <sub>8</sub>	<sup>1</sup> / <sub>2</sub>	<sup>5</sup> / <sub>8</sub>		
Location of Installation	-	-	upper and lower flute	upper and lower flute	lower flute	lower flute	
Minimum effective embedment depth	$h_{ef}$	inches	<sup>1</sup> / <sub>2</sub>	2	<sup>3</sup> / <sub>4</sub>	<sup>2</sup> / <sub>4</sub>	<sup>4</sup> / <sub>4</sub>
Characteristic pullout strength, uncracked concrete over metal deck	$N_{p, deck, uncr}^1$	lbf	2,170	2,515	5,285	3,365	6,005
Characteristic pullout strength, cracked concrete over metal deck	$N_{p, deck, cr}^1$	lbf	1,650	1,780	4,025	2,405	5,025
Characteristic shear strength, concrete over metal deck	$V_{sa, deck}^1$	lbf	1,640 <sup>3</sup>	2,200	3,790	2,890	6,560
Reduction factor for pullout strength in tension, Condition B	$\phi$	-	0.65	0.65		0.65	
Reduction factor for steel strength in shear, Condition B	$\phi$	-	0.60	0.65		0.65	

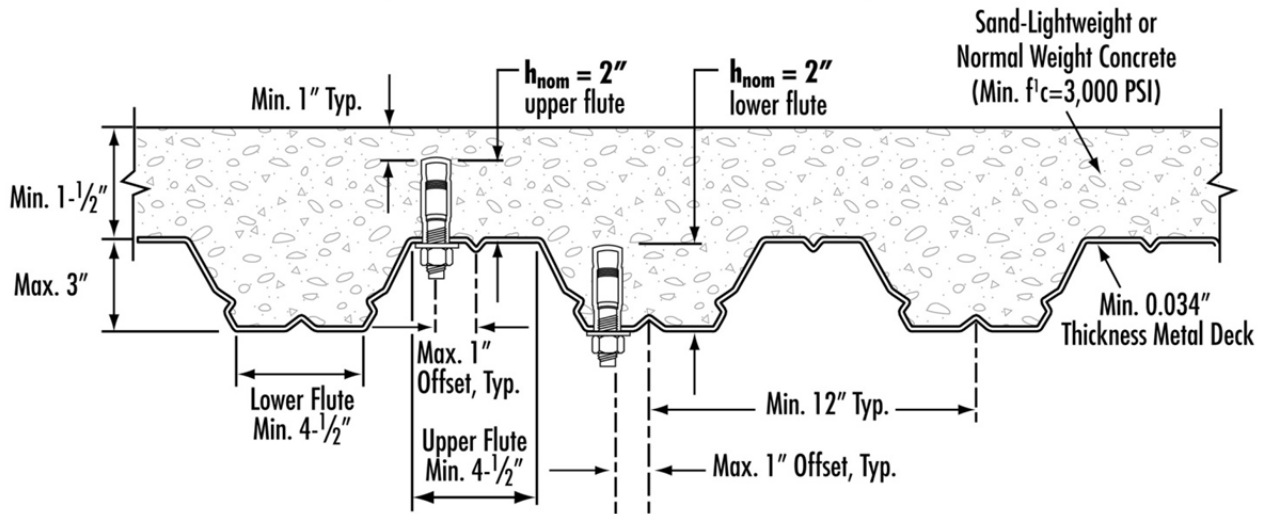
For **SI**: 1 inch = 25.4 mm, 1 lbf = 4.45 N.

<sup>1</sup> Values for  $N_{p, deck}$  and  $V_{sa, deck}$  apply to sand-lightweight concrete having a minimum concrete compressive strength,  $f_c$ , of 3,000 psi.

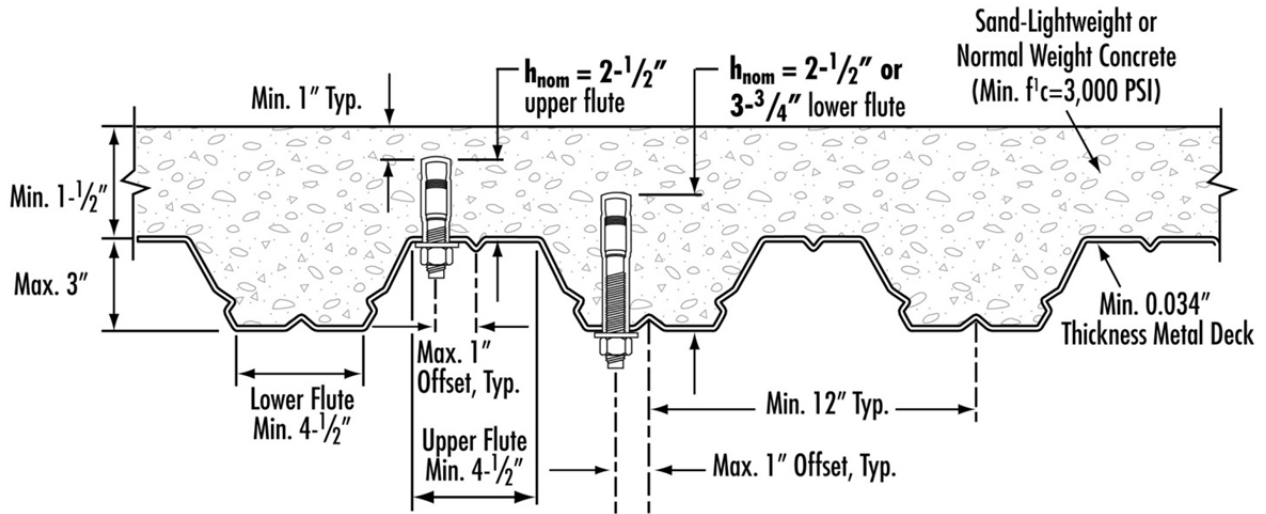
<sup>2</sup> All values of  $\phi$  apply to the load combinations of IBC Section 1605.2 or ACI 318 Section 9.2. If the load combinations of Appendix C are used, the appropriate value of  $\phi$  must be determined in accordance with ACI 318-11 D.4.4 (ACI 318-08 and -05 D.4.5). For installations where reinforcement that complies with ACI 318 Appendix D requirements for Condition A is present, the appropriate  $\phi$  factor must be determined in accordance with ACI 318-11 D.4.3 (ACI 318-08 and -05 D.4.4).

<sup>3</sup> For seismic applications according to Section 4.10.3 of this report multiply the value of  $V_{sa, deck}$  by 0.84.

**Nominal Anchor Diameter = 3/8"**



**Nominal Anchor Diameter = 1/2"**



**Nominal Anchor Diameter = 5/8"**

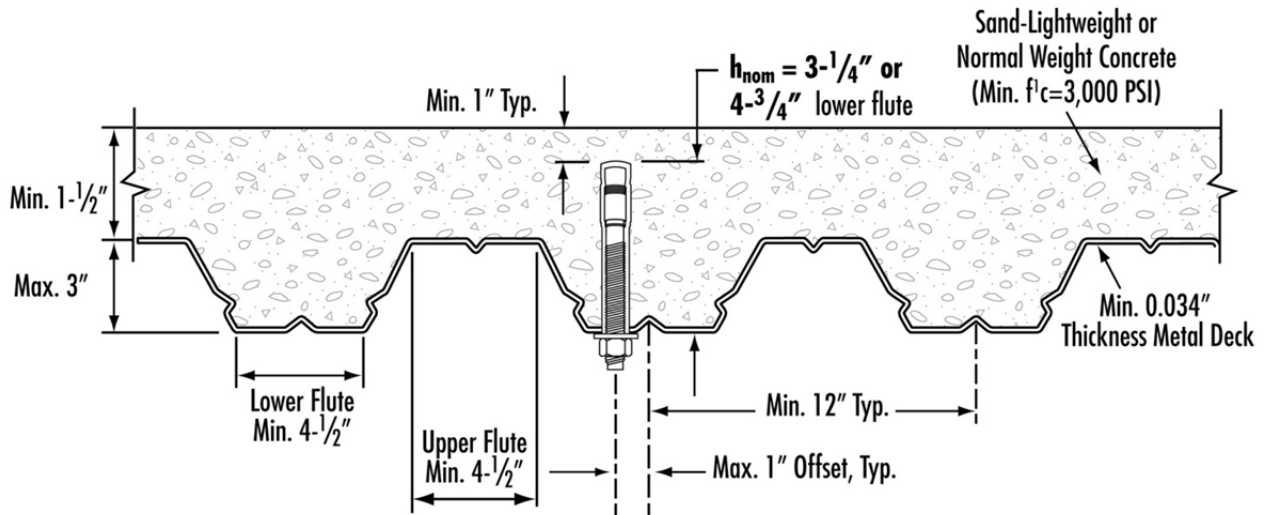


FIGURE 7—ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHOR AND OVERHEAD TRUBOLT+ WEDGE ANCHOR LOCATED IN THE SOFFIT OF CONCRETE OVER STEEL DECK FLOOR AND ROOF ASSEMBLIES (1 inch = 25.4 mm)

**ICC-ES Evaluation Report****ESR-2427 FBC Supplement\***

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techsupport@itwccna.com**EVALUATION SUBJECT:****ITW RED HEAD CARBON STEEL TRUBOLT+ WEDGE ANCHORS, STAINLESS STEEL TRUBOLT+ WEDGE ANCHORS  
AND CARBON STEEL OVERHEAD TRUBOLT+ WEDGE ANCHORS FOR CRACKED AND UNCRACKED CONCRETE****1.0 REPORT PURPOSE AND SCOPE****Purpose:**

The purpose of this evaluation report supplement is to indicate that ITW Red Head Trubolt+ Wedge Anchors and OVERHEAD Trubolt+ Wedge Anchors for Cracked and Uncracked Concrete, recognized in ICC-ES master evaluation report ESR-2427, has also been evaluated for compliance with the codes noted below.

**Applicable code editions:**

- 2010 *Florida Building Code—Building*
- 2010 *Florida Building Code—Residential*

**2.0 CONCLUSIONS**

The ITW Red Head Trubolt+ Wedge Anchors and OVERHEAD Trubolt+ Wedge Anchors for Cracked and Uncracked Concrete, described in Sections 2.0 through 7.0 of the master evaluation report ESR-2427, comply with the 2010 *Florida Building Code—Building* and the 2010 *Florida Building Code—Residential*, provided the design and installation are in accordance with the *International Building Code*® provisions noted in the master report and the following provisions apply:

- Design wind loads must be based on Section 1609 of the 2010 *Florida Building Code—Building* or Section 301.2.1.1 of the 2010 *Florida Building Code—Residential*, as applicable.
- Load combinations must be in accordance with Section 1605.2 or Section 1605.3 of the 2010 *Florida Building Code—Building*, as applicable.
- The modifications to ACI 318 as shown in the 2009 IBC Sections 1908.1.9 and 1908.1.10, as noted in 2009 IBC Section 1912.1, do not apply to the 2010 *Florida Building Code*.

Use of the ITW Red Head Trubolt+ Wedge Anchors and OVERHEAD Trubolt+ Wedge Anchors for Cracked and Uncracked Concrete for compliance with the High-Velocity Hurricane Zone provisions of the 2010 *Florida Building Code—Building* and the 2010 *Florida Building Code—Residential* has not been evaluated, and is outside the scope of this supplemental report.

For products falling under Florida Rule 9N-3, verification that the report holder's quality assurance program is audited by a quality assurance entity approved by the Florida Building Commission for the type of inspections being conducted is the responsibility of an approved validation entity (or the code official when the report holder does not possess an approval by the Commission).

This supplement expires concurrently with the master report reissued November 2013, revised May 2014.

**\*Revised May 2014**