L. Roberto Lomas P.E.

1432 Woodford Rd. Lewisville, NC 27023 434-688-0609

rllomas@lrlomaspe.com

Test Report: N/A

Product: Single door 3'x8' (Wood Frame)

Scope:

This analysis provides calculations, quantities, and spacing requirements for installing product to substrate, and it applies only to the product described herein. These calculations comply with requirements of the Florida Building Code. Anchor capacity in shear condition:

Solid members w/ & w/out gap:

۵.	With threads preser	nt in shea	r plane										
		Fasten	er type:	#10 wood	screw		(NDS 201	2, TR12)					
	1	Nominal di	ameter:	D:	0.190	in			Gap	g:	0.0000 in		
		Root di	ameter:	Dr:	0.152	in			Moment arm:		0.0000 in		
	Minimum required penetration: p:			1.140	in	S	icrew bendir	ng yield strength:	F _{уb} =	80,000 psi			
	Side member: Douglas Fir-Larch (G=0.50)					; =0.50)	i0) Main member			Spruce-P	ine-Fir (G=0.4	2)	
	Side member thickness: $t_s =$				1.000	1.000 in		Main member thickness: Main member dowel bearing strength:		t _m = F _{em} =	1.500 in		
	Side member dowel bearing strength: F_{es} =			4,650 psi		Main men	3,350 psi						
	Side member dow	el bearing	length:	l _s =	1.000	in	Main m	nember dowe	el bearing length:	I _m =	1.140 in		
Mod	e I _m	Mode	Is	_	Mode	2 II	Mode	III _m	Mode	e III _s		Mode	2 IV
qm =	636.5 lbs/in	qs =	884	lbs/in	A :	0.0007	A:	0.00096	A:	0.00107		A :	0.0014
P =	725.61 lbs	P =	884	lbs	B:	1.07	B:	0.57	B:	0.5		B:	0.000
K _D =	2.400	K _D =	2.400		<i>C</i> :	-427.67	<i>C</i> :	-253.62	<i>C</i> :	-267.7		C:	-93.6
∠ _m =	302 lbs	∠ _s =	368	lbs	P =	331 lbs	Ms =	46.8 in	-lbs Mm =	46.8	in-lbs		0(0 H
	His Nationality	7	110	U	κ _D =	2.400	P =	297 Ib	s P=	319	IDS	Р= И -	263 Ibs
	Min. Design value:	Z=	110	IDS	Ζ=	138 IDS	κ _D =	2.400	K _D =	2.400		K _D =	2.400
	Duration Factor: Allowable De	sign Value	1.6 • (ZC _D):	Z'=	175	lbs/anchor	Ζ=	124 Ib	s ∠=	133	Ibs	Ζ=	110 lbs
Solid men	nbers w/ & w/out ga	p:											
۵.	With threads preser	nt in shea	r plane										
		Fasten	er type:	#10 wood	screw		(NDS 201	2, TR12)					
	1	Nominal di	ameter:	D:	0.190	in			Gap:	g:	0.0000 in		
		Root di	ameter:	Dr:	0.152	in			Moment arm:		0.0000 in		
	Minimum requ	uired pene	tration:	p:	1.140	in	S	icrew bendir	ng yield strength:	F _{уb} =	80,000 psi		
		Side r	nember:	Douglas Fir	-Larch (6	; =0.50)			Main member:	Steel str	ap/clip		
	Side m	nember th	ickness:	† _s =	1.000	in		Main m	ember thickness:	† _m =	0.048 in		
	Side member dowel	bearing st	rength:	F _{es} =	4,650	psi	Main men	nber dowel b	pearing strength:	F _{em} =	61,850 psi		
	Side member dow	el bearing	length:	l _s =	1.000	in	Main m	nember dowe	el bearing length:	I _m =	1.140 in		
Mod	e I _m	Mode	Is	_	Mode	2 II	Mode	III _m	Mode	e III _s		Mode	2 IV
qm =	11752 lbs/in	qs =	884	lbs/in	A :	0.0003	A:	0.00059	A:	0.00033		A:	0.0006
P =	13397 lbs	P =	884	lbs	B:	1.07	B:	0.57	B:	0.5		B:	0.000
K _D =	2.400	K _D =	2.400		C:	-4038.9	С:	-3864.9	<i>C</i> :	-267.7		C:	-93.6
∠ _m =	5582 lbs	Z _s =	368	lbs	P =	2287 lbs	Ms =	46.8 in	-lbs Mm =	46.8	in-lbs		
		-	44.0		κ _D =	2.400	P =	2126 lb	s P=	420	lbs	P =	392 lbs
	Min. Design value:	Ζ=	163	IDS	Ζ=	953 lbs	κ _D =	2.400	κ _D =	2.400		κ _D =	2.400
	Duration Factor:	C _D =	1.6				Z=	886 Ib	s Z=	175	lbs	Z=	163 lbs
	Allowable De	sign value	: (ZC _D):	Z'=	262	bs/anchor							
	Fastener type: 1/4" ITW Tapcon						N	.O.A. 16-1222.06					
	Substrate: Hollow bloc				k		Mi	nimum embe	:dment:	1.25	in		
	Edge distance: 4.00 ir				ı	Tabula	ated shear desi	ign value:	Z = 202	lbs			
	Edge distance: 2.00 in			ı	Tabula	ated shear desi	ign value:	Z = 161	lbs				
	Actual edge distance: 2.50			2.50 ir	ı		Reductio	n factor:	0.85				111111
	Spacing:			4.00 ir	ı	Tabula	ated shear desi	ign value:	Z = 202	lbs		111.	R. 1
	Spacing:			2.00 ir	ı	Tabula	ated shear desi	ign value:	Z = 164	lbs	1	113	
		Actual	spacing:	3.00 ir	ı		Reductio	n factor:	0.91		1	~	CEN.
	Allowable Design Value (Zf _{AN}):			Z''=	155	lbs/anchor					1	. ~	
-	Minimum anchor capacity:			155	lbs/anchor					=**		10. 625	

Note: Anchors with the least capacity is used for calculations to qualify anchors with higher capacity.

Anchor calculations, minimum required anchors

36.38	
1	
	95.25

Design pressure: 70.0 psf										
	Zone	Area (ft²)	Load (Ibs)	Ind. (in)	Max.					
					0.C. (in)	Cap. (lbs)	Qty	Load (Ibs)	Result	
	A ₁	2.3	161	N/A	N/A	155	2	80	OK	
	A ₂	9.7	681	6.00	18.00	155	6	114	OK	



Luis R. Lomas P.E. FL No.: 62514 5/2/2017

L. Roberto Lomas P.E.

1432 Woodford Rd. Lewisville, NC 27023 434-688-0609 rllomas@Irlomaspe.com

Anchor Locations:



Installation instructions:

- 1. FOR ANCHORING THROUGH FRAME INTO WOOD FRAMING OR 2X BUCK USE #10 WOOD SCREWS WITH SUFFICIENT LENGTH TO ACHIEVE A 1 1/4" MINIMUM EMBEDMENT INTO SUBSTRATE WITH 1/2" MINIMUM EDGE DISTANCE. LOCATE ANCHORS AS SHOWN BELOW.
- 2. FOR ANCHORING THROUGH FRAME INTO MASONRY/CONCRETE USE 3/16" TAPCONS WITH SUFFICIENT LENGTH TO ACHIEVE A 1 1/4" MINIMUM EMBEDMENT INTO SUBSTRATE WITH 2 1/2" MINIMUM EDGE DISTANCE. LOCATE ANCHORS AS SHOWN BELOW.
- 3. FOR ANCHORING THROUGH FRAME INTO METAL STRUCTURE USE #10 SMS OR SELF DRILLING SCREWS WITH SUFFICIENT LENGTH TO ACHIEVE 3 THREADS MINIMUM BEYOND STRUCTURE INTERIOR WALL WITH 1/2" MINIMUM EDGE DISTANCE. LOCATE ANCHORS AS SHOWN BELOW.
- 4. ALL FASTENERS TO BE CORROSION RESISTANT.
- 5. INSTALLATION ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH ANCHOR MANUFACTURER'S INSTALLATION INSTRUCTIONS AND ANCHORS SHALL NOT BE USED IN SUBSTRATES WITH STRENGTHS LESS THAN THE MINIMUM STRENGTH SPECIFIED BELOW: A. WOOD: MINIMUM SPECIFIC GRAVITY OF G=0.42
 - B. CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI.

C. MASONRY: HOLLOW/FILLED BLOCK PER ASTM C90 WITH Fm=2,000PSI MINIMUM. D. METAL STRUCTURE: STEEL 18GA (.048") FY=33KSI/FU=52KSI OR ALUMINUM 6063-T5 FU=30KSI .052" THICK MINIMUM

6. ANCHOR LOCATIONS SHOWN IN THIS DOCUMENT ARE THE MINIMUM REQUIRED FOR THE DESCRIBED PRODUCT EXPOSED AT THE DESIGN PRESSURE INDICATED HEREIN.

Luis R. Lomas P.E. FL No.: 62514 5/2/2017